



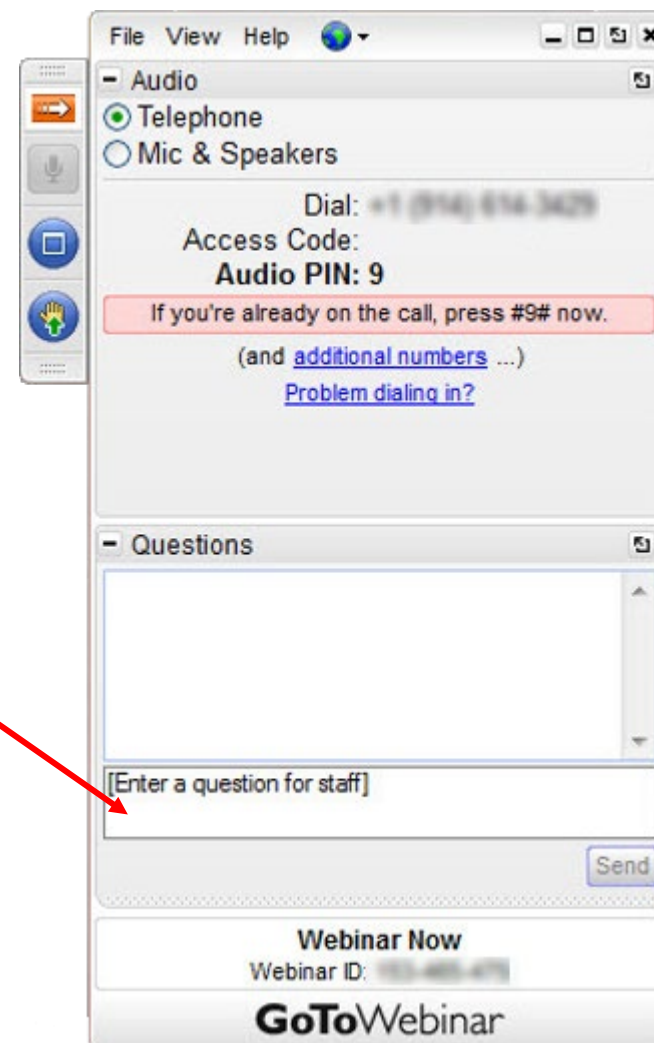
Itasca Constitutive Model for Advanced Strain Softening *IMASS*

Background and Applied Examples

Ehsan Ghazvinian (Geomechanics Engineer)

Agenda

- Brief introduction to strain-softening constitutive models
- Theory of *IMASS*
- Examples
- Questions and answer session
 - ❖ Please type any questions in the question panel and we'll answer them at the end of the webinar



Introduction

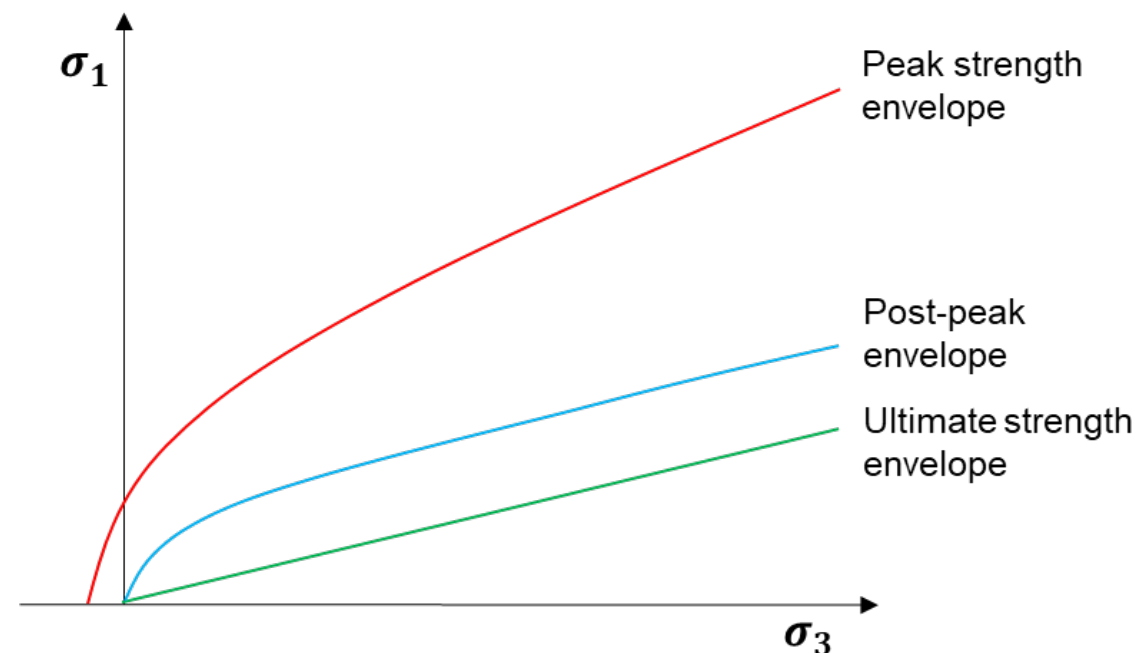
A numerical model that represents the damage around an excavation, slope or caving process must account for the progressive failure and disintegration of the rock mass from an intact/jointed condition to a bulked material. Four critical factors that control the overall behavior of the rock mass matrix during this process are:

- Cohesion and Tension Weakening and Frictional Strengthening
- Post Peak Brittleness
- Modulus Softening
- Dilatational Behavior

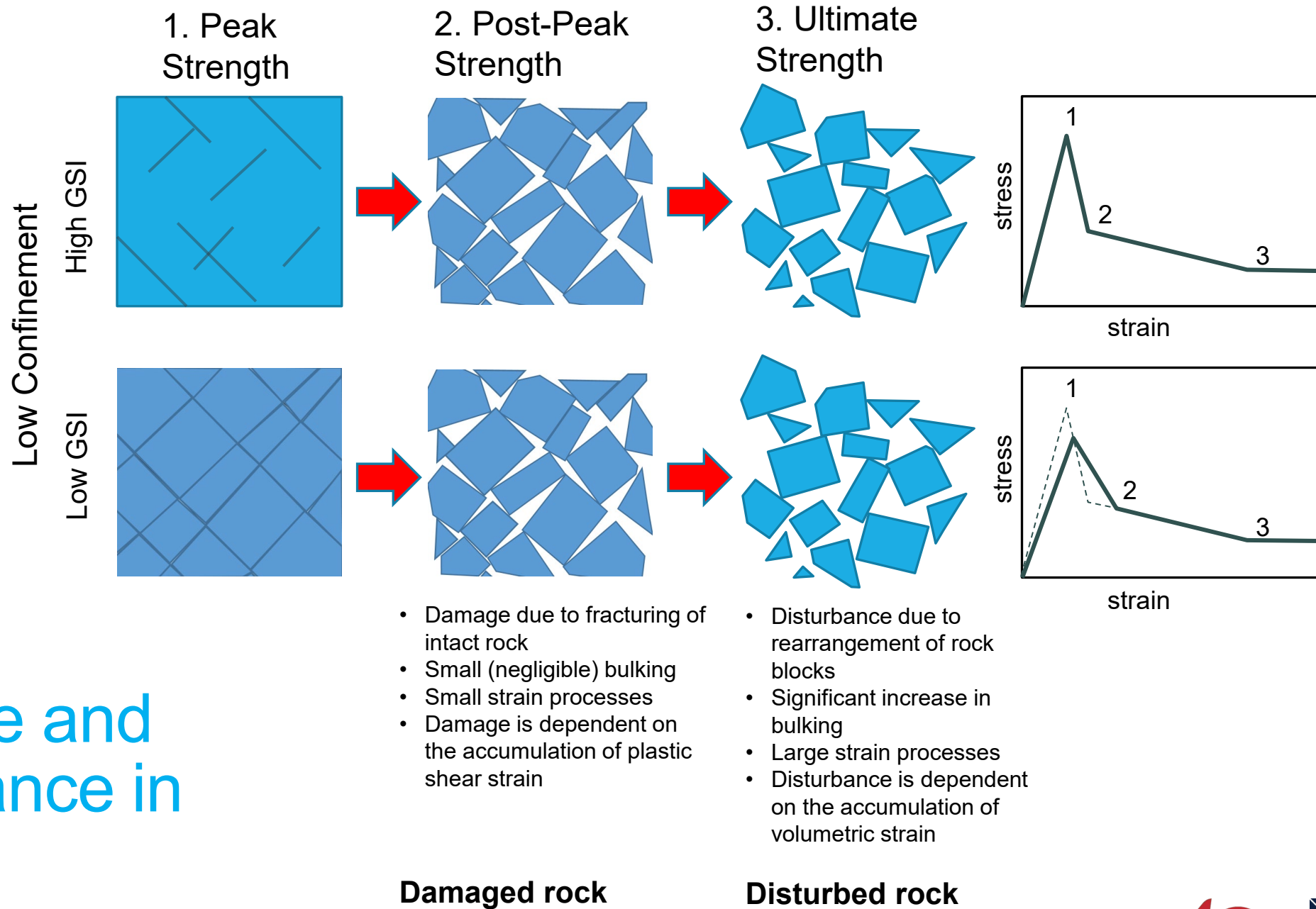
This overall process – loading the rock mass to its peak strength, followed by a post-peak reduction in strength to some residual level with increasing strain – often is termed a “strain-softening” process and is the result of strain-dependent material properties.

IMASS constitutive model

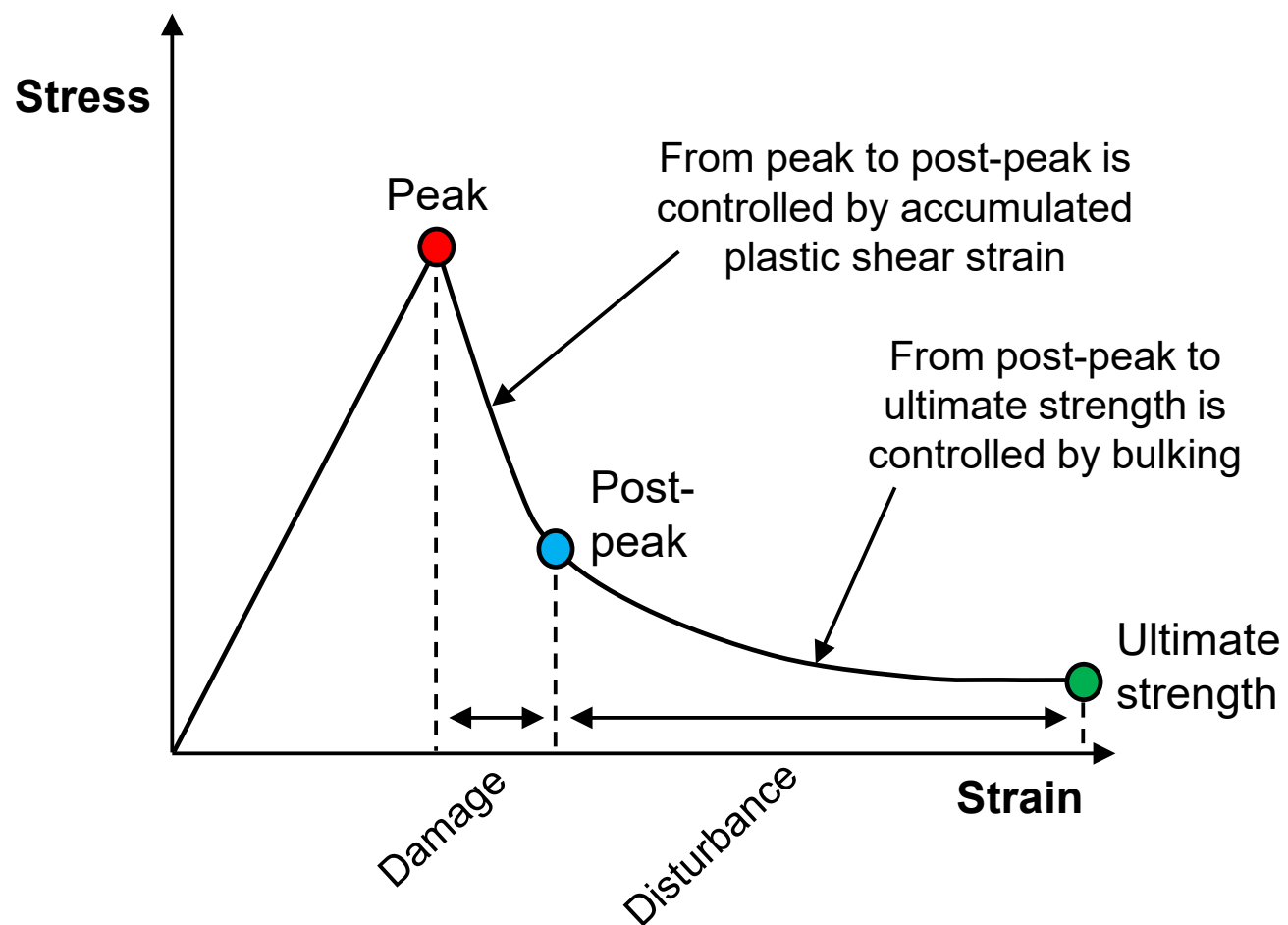
- The Itasca Constitutive Model for Advanced Strain Softening (*IMASS*) is a successor to the original *CaveHoek* constitutive model (first appearing in 2010)
- In terms of strength envelopes, *CaveHoek* is characterized by two bounding yield surfaces (peak and residual)
- After many successful projects and new discoveries about brittle rock behavior, a new strain softening model has been created (Itasca Model for Advanced Strain Softening)
- *IMASS* contains two softening (residual) yield surfaces



Damage and disturbance in IMASS



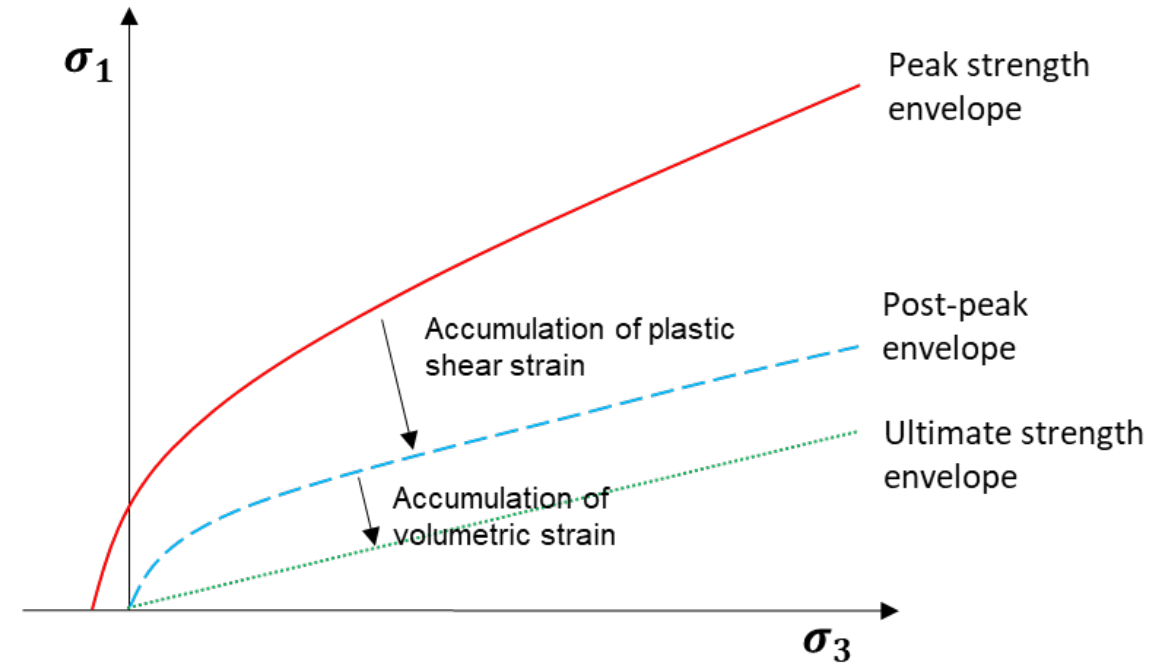
Conceptual stress-strain curve



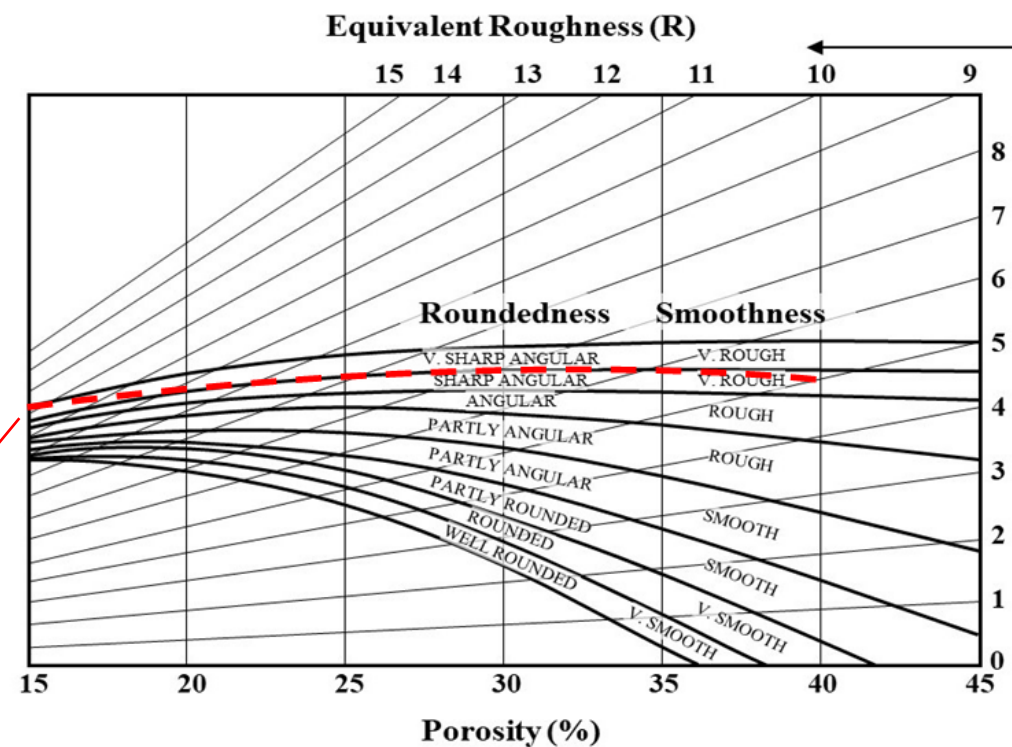
Strength weakening in *IMASS*

- *IMASS* constitutive model is defined by three Hoek-Brown strength envelopes
- The GSI, m_i , and UCS parameters control the shape of the peak Hoek-Brown envelope (Hoek et al., 2002)
- The Hoek-Brown parameters of the residual strength envelopes are calculated in order to approximate Barton & Kjaernsli (1981) shear strength for rockfill material:

$$\tau = \sigma_n \tan \left(R \cdot \log \left(\frac{S}{\sigma_n} \right) + \phi_b \right)$$



- Extrapolated
to 0%
porosity



$$\tau = \sigma_n \tan \left(R. \log \left(\frac{S}{\sigma_n} \right) + \phi_b \right)$$

Residual strength envelopes in *IMASS*

When the shear strength from the Barton & Kjaernsli (1981) equation is converted to a strength envelope in $\sigma_1 - \sigma_3$ space, it can be approximated by a Hoek-Brown envelope with the following parameters:

$$s = 0$$

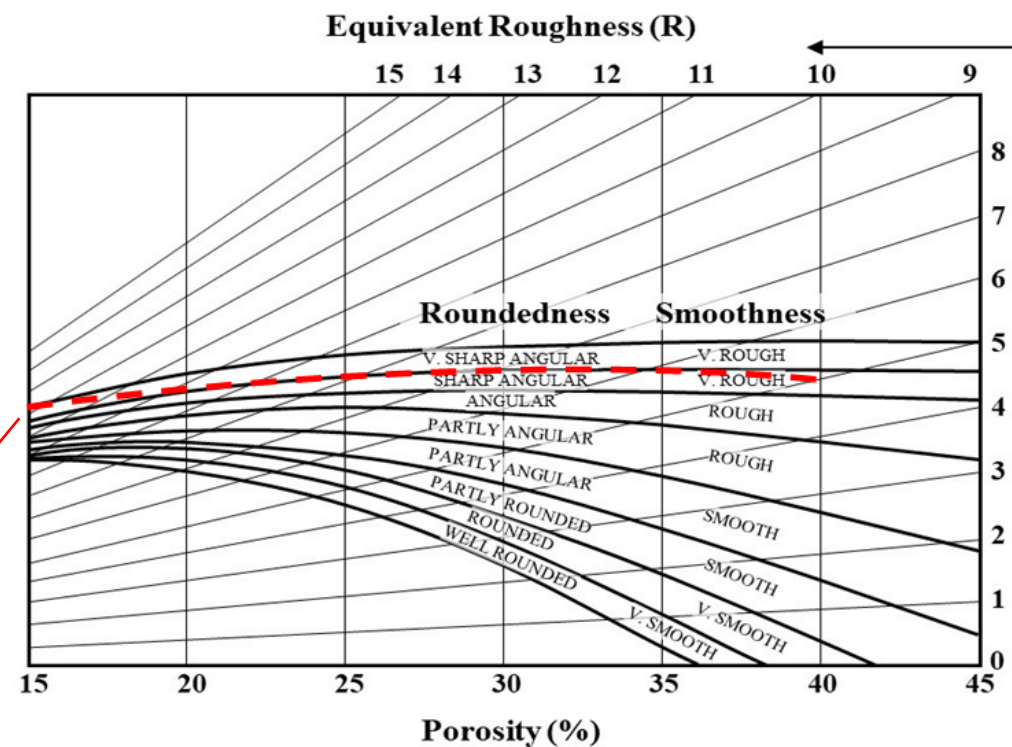
$$a = 0.6 + \frac{\text{porosity}}{\text{porosity}_{\max}} \times [(1 - 0.075 \times ri) - 0.6]$$

$$m_b = 0.1614 \times e^{0.0836 \times \text{in_weak_phib}}$$

where,

- ri is the roundedness index (with $ri = 0$ for partly rounded/smooth blocks, $ri = 1$ for angular/rough blocks, and $ri = 2$ for very sharp, angular/very rough blocks).
- in_weak_phib is equivalent to ϕ_b (in degrees and default = 30 deg)

Extrapolated
to 0%
porosity



Residual strength envelopes in *IMASS*

When the shear strength from the Barton & Kjaernsli (1981) equation is converted to a strength envelope in $\sigma_1 - \sigma_3$ space, it can be approximated by a Hoek-Brown envelope with the following parameters:

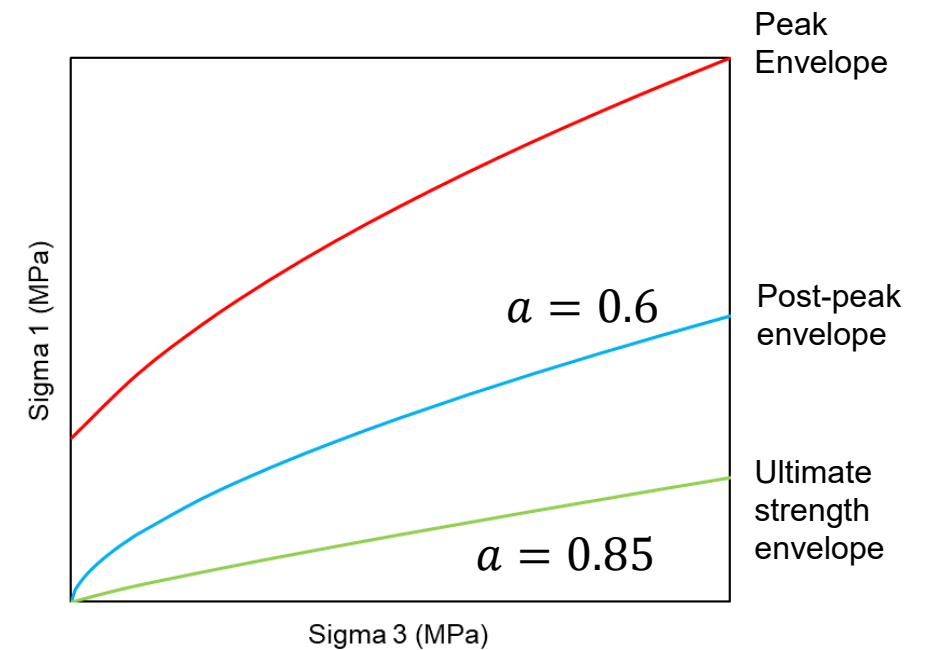
$$s = 0$$

$$a = 0.6 + \frac{\text{porosity}}{\text{porosity}_{\max}} \times [(1 - 0.075 \times ri) - 0.6]$$

$$m_b = 0.1614 \times e^{0.0836 \times \text{in_weak_phib}}$$

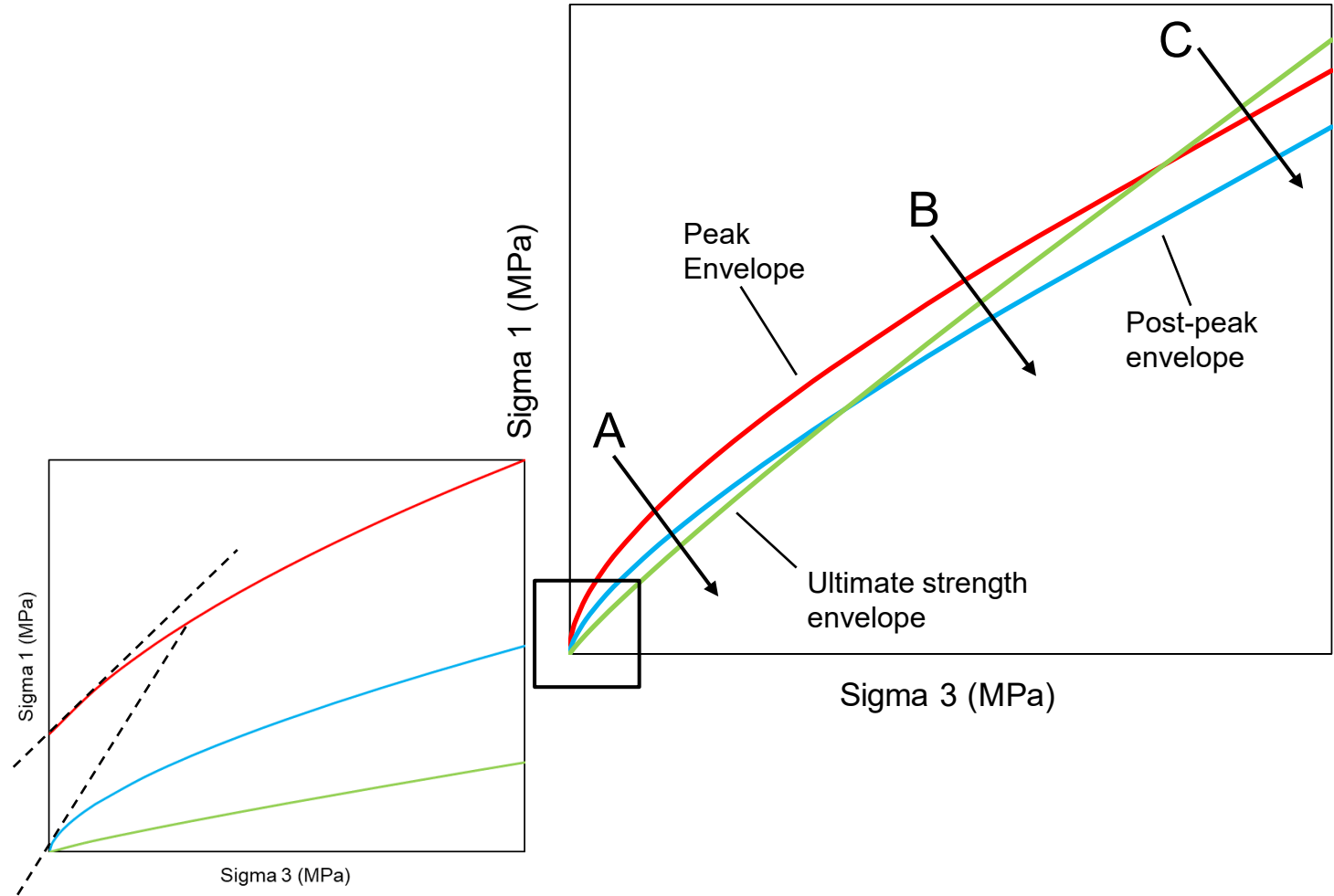
The Hoek-Brown approximation of Barton & Kjaernsli (1981) shear strength criteria for rockfill that is implemented in *IMASS* assumes formation and interaction of very sharp, angular and very rough fragments during the course of bulking, from porosity 0% to 40%.

$$a = 0.6 + \left(\frac{VSI}{0.67} \times 0.25 \right)$$

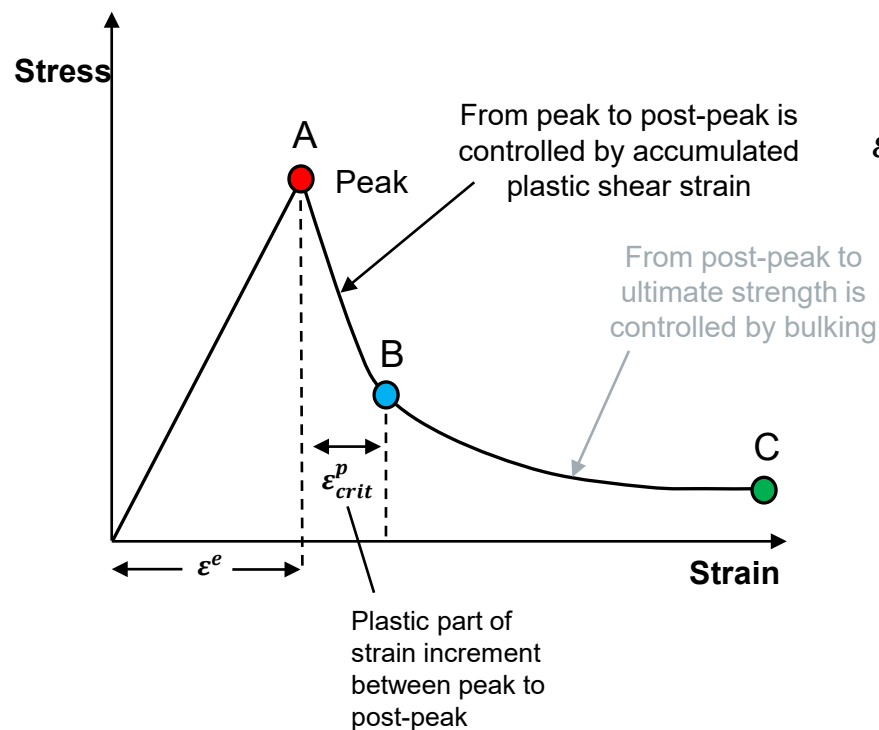
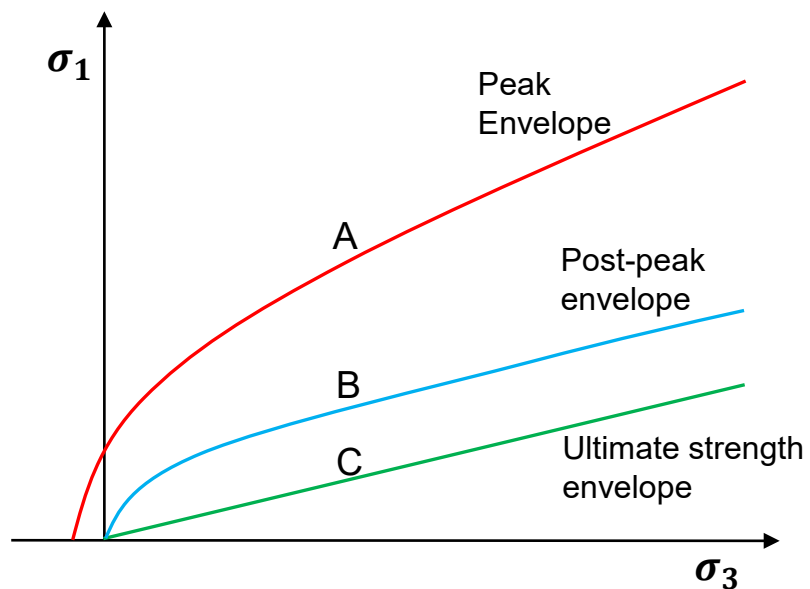


Characteristics of *IMASS* residual envelopes

- Zero or near zero apparent cohesion and high friction angle at low confinement for the post-peak envelope
- Lower friction angle at low confinement for the ultimate strength envelope
- Both post-peak and ultimate strength envelopes continue to use peak Hoek-Brown envelope at higher confinement above brittle-ductile transition



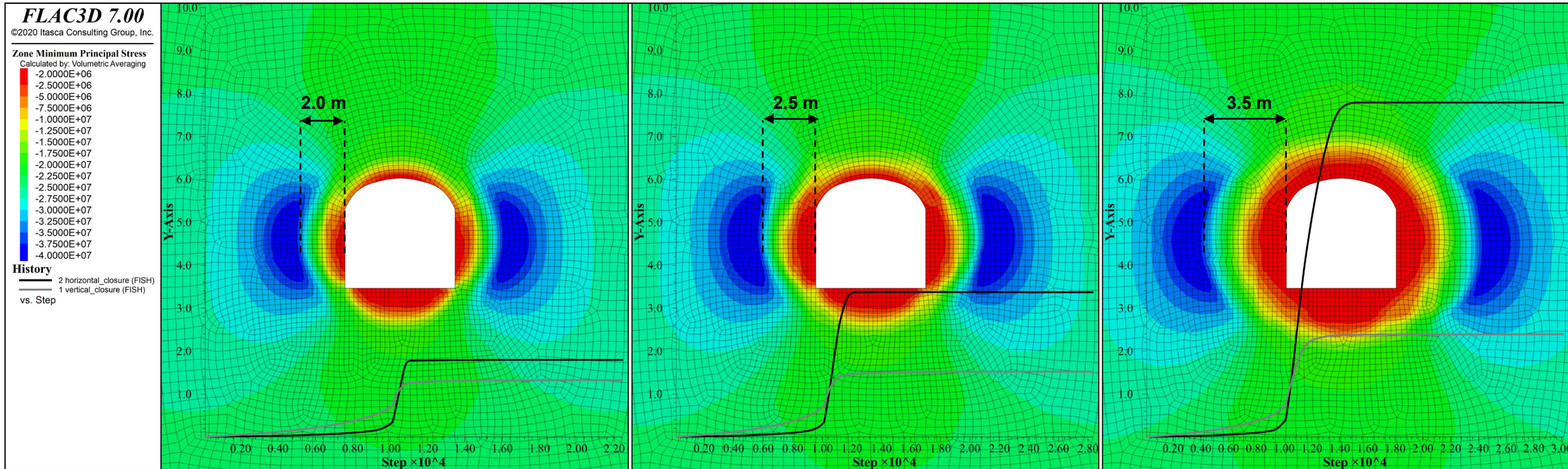
Post-peak brittleness



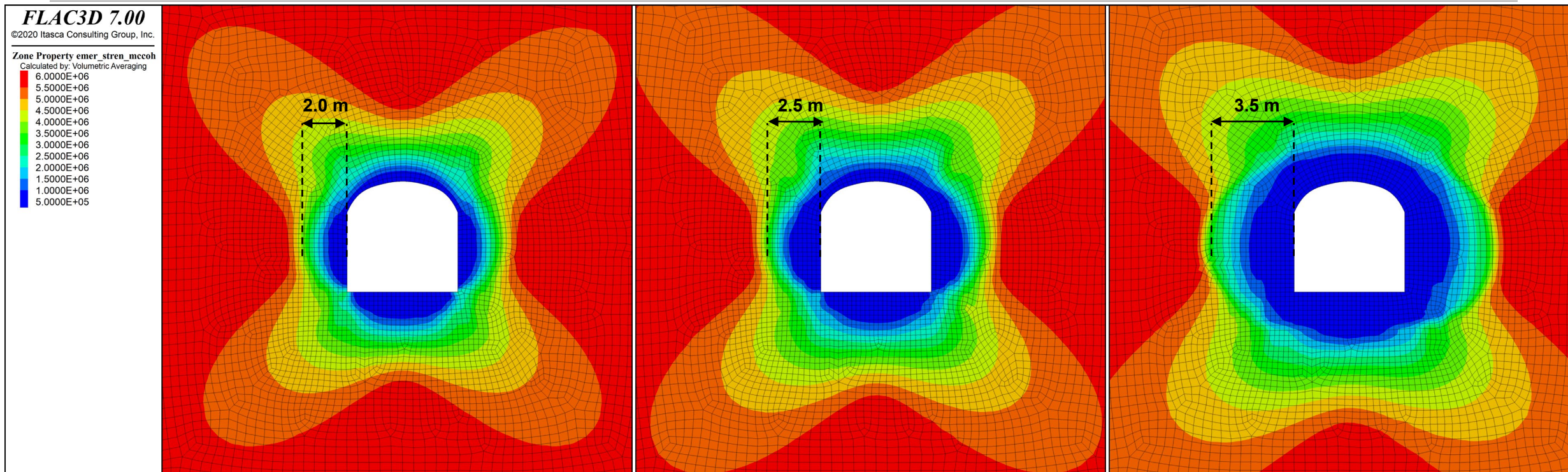
$$\epsilon_{crit}^p = \frac{12.5 - 0.125 * GSI}{100 * d}$$

(Lorig & Pierce, 2000)

Critical Strain sensitivity



Cohesion weakening



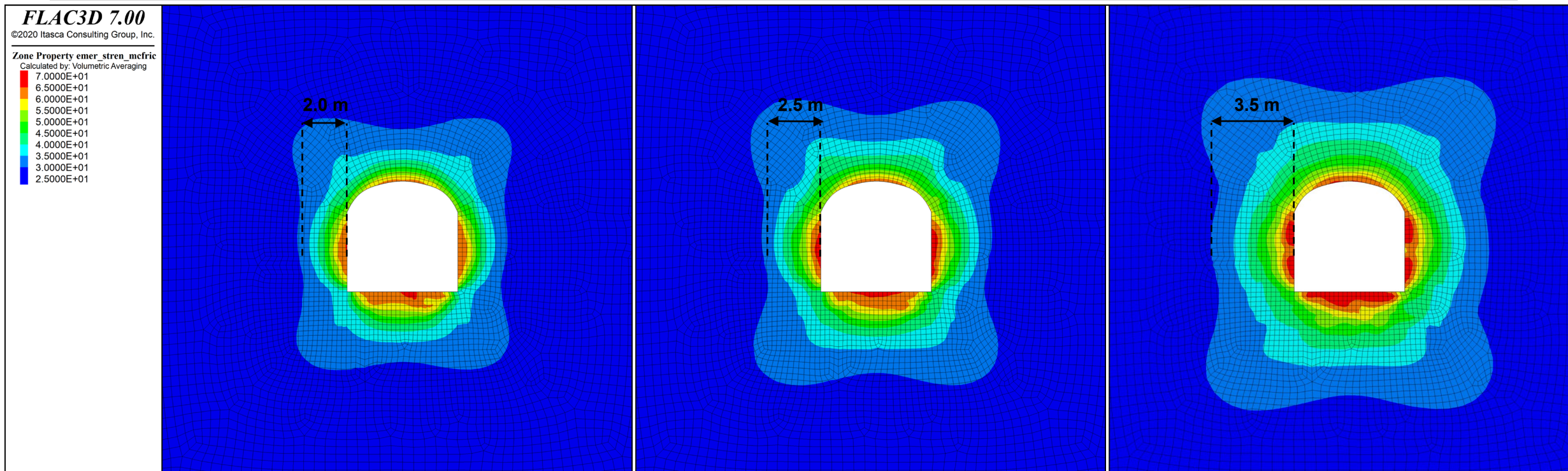
Multiplier ecrit = 1.0
ecrit ~ 30%
Vertical tunnel closure ~ 1%
Horizontal tunnel closure ~ 2%

Multiplier ecrit = 0.1
ecrit ~ 3%
Vertical tunnel closure ~ 1%
Horizontal tunnel closure ~ 3.5%

Multiplier ecrit = 0.01
ecrit ~ 0.3%
Vertical tunnel closure ~ 2%
Horizontal tunnel closure ~ 8%



Frictional strengthening

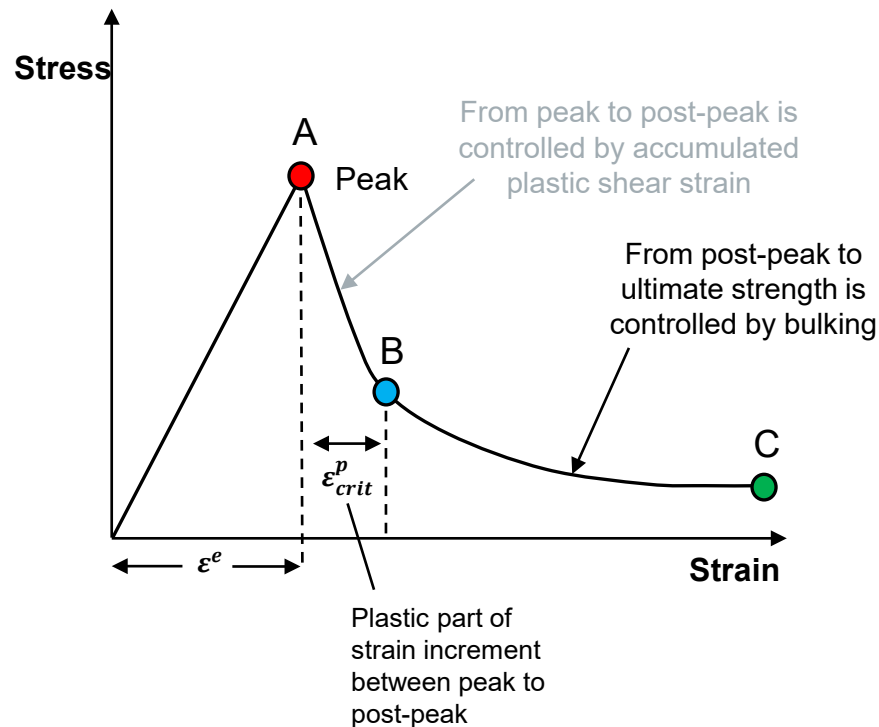
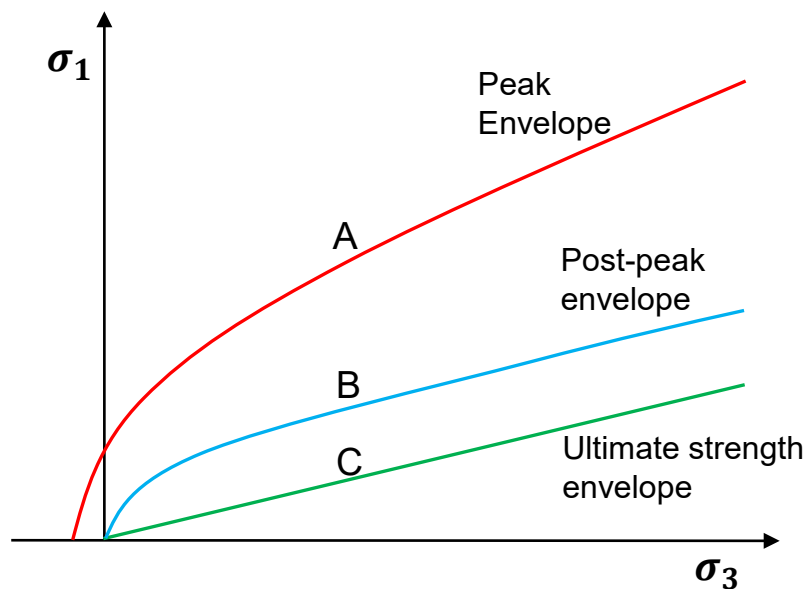


Multiplier ecrit = 1.0
ecrit ~ 30%
Vertical tunnel closure ~ 1%
Horizontal tunnel closure ~ 2%

Multiplier ecrit = 0.1
ecrit ~ 3%
Vertical tunnel closure ~ 1%
Horizontal tunnel closure ~ 3.5%

Multiplier ecrit = 0.01
ecrit ~ 0.3%
Vertical tunnel closure ~ 2%
Horizontal tunnel closure ~ 8%

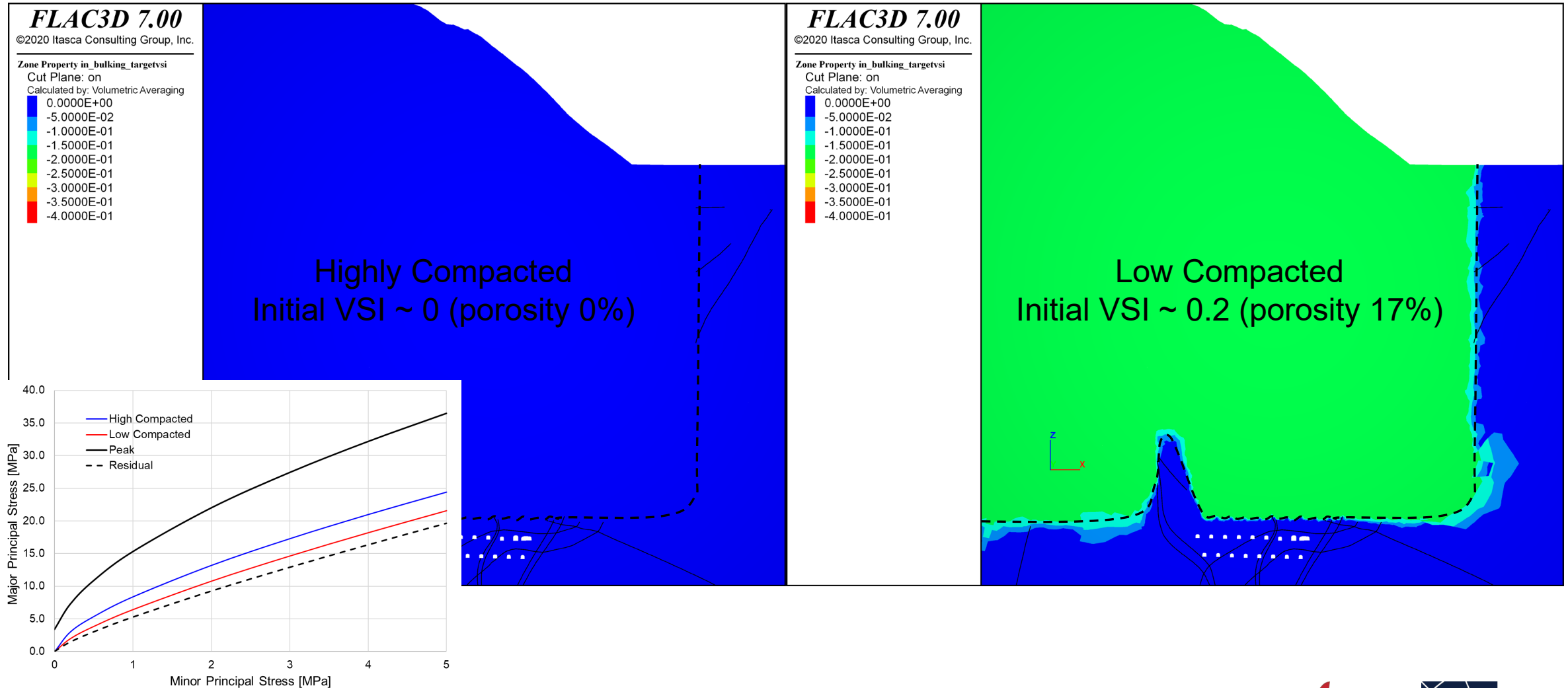
Porosity-dependent softening/weakening



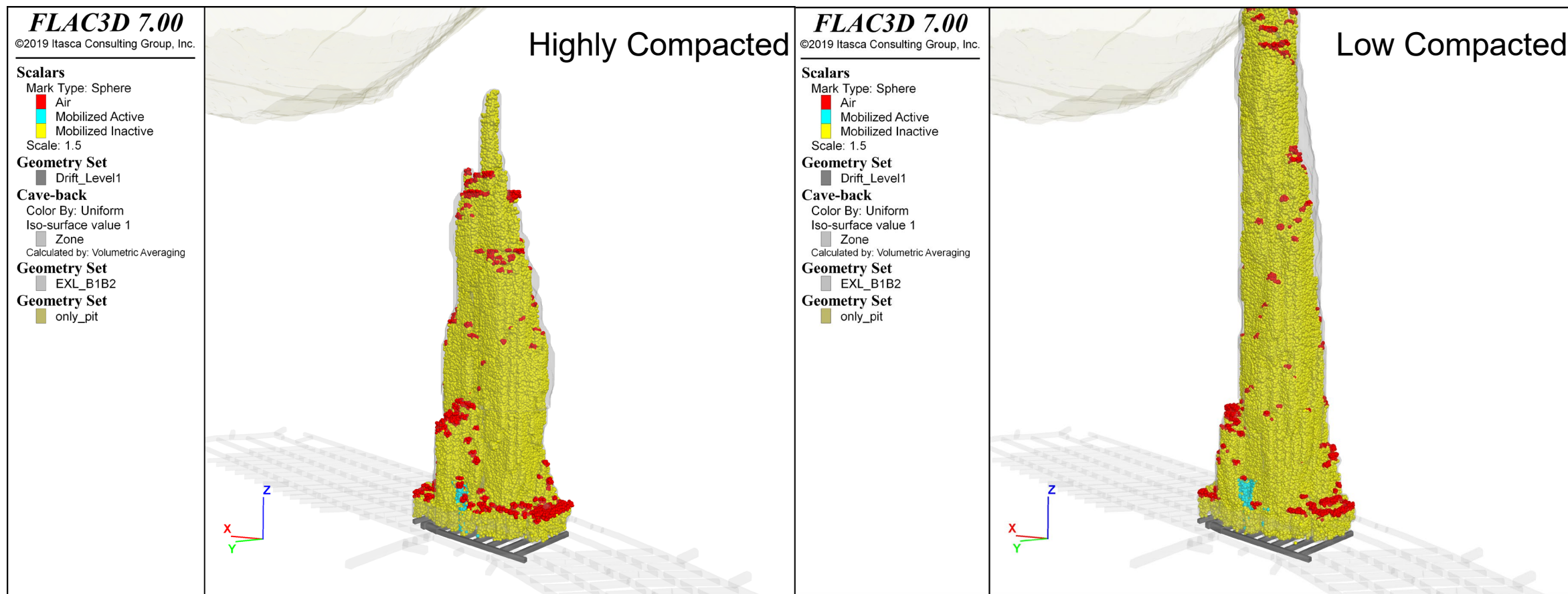
$$B = \frac{\Delta V}{V_i} = \frac{n}{1 - n}$$

- Residual strength can weaken and strengthen between post-peak and ultimate strength envelopes as a function of porosity
- This would allow for capturing strength gain in material due to recompaction

Two cases of initial compaction



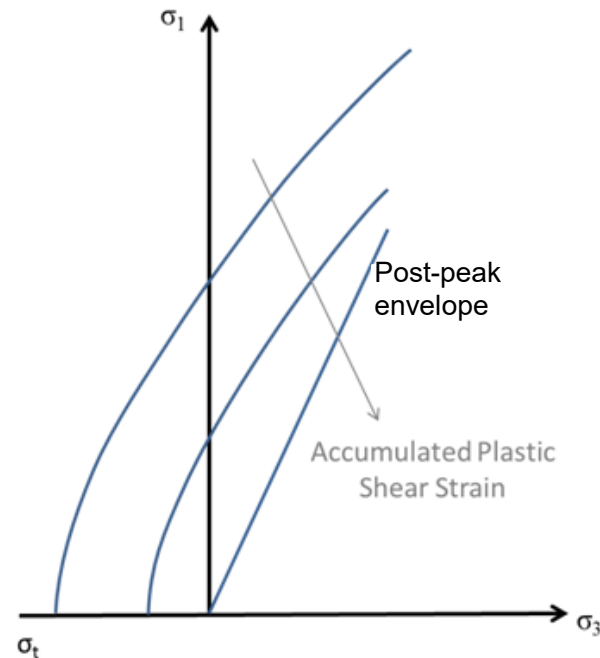
Cave propagation



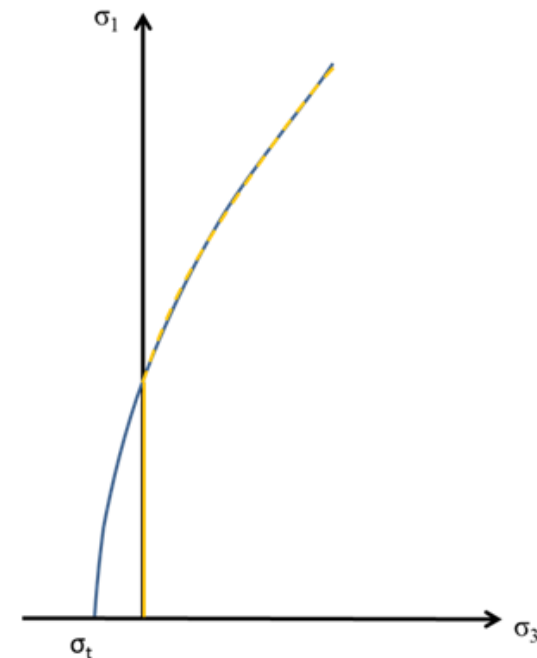
Tension weakening in *IMASS*

The mechanism of independent tensile softening is implemented in *IMASS*, i.e., in addition to softening tension and cohesion at the same rate based on plastic shear strain, tension is allowed to soften independently of the cohesion in the instance of a tensile yield state within a zone.

Alternatively, a table that correlates tension weakening to accumulated plastic tensile strain can be assigned to zones to control the tension weakening.



Tensile strength degradation occurring with accumulated plastic shear strain



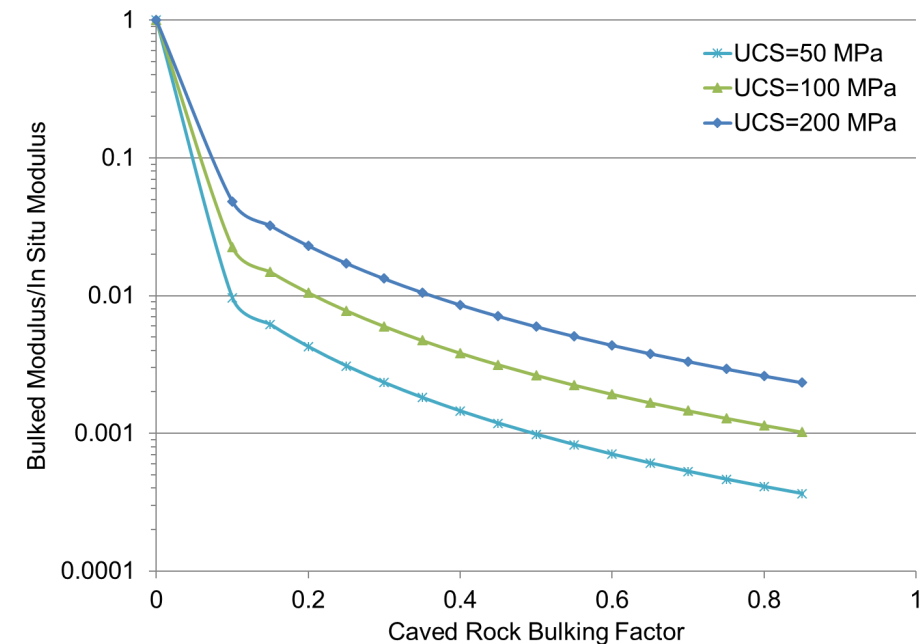
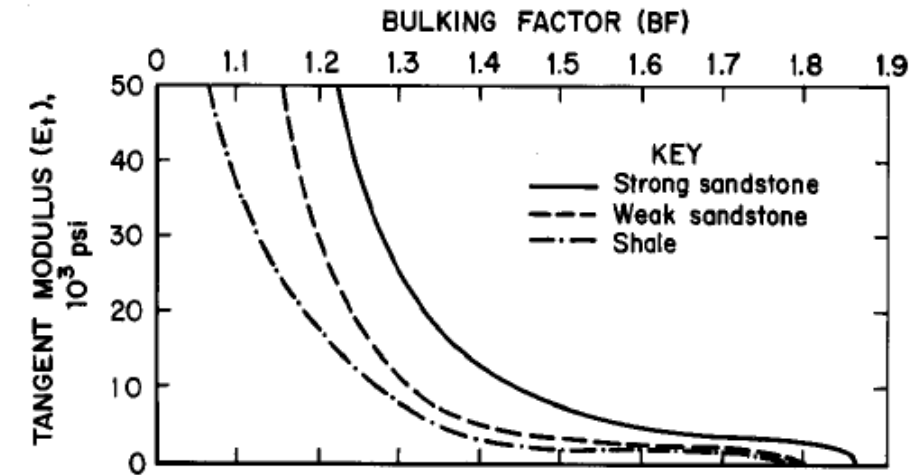
Sudden loss of tensile strength during tensile failure (perfectly brittle in tension)

Modulus softening

The rock mass Young's modulus (E_{rm}) can be estimated from the intact Young's modulus (E_i) and GSI using Hoek and Diederichs' (2006) equation:

$$E_{rm} = E_i \left(0.02 + \frac{1}{1 + e^{\frac{60 - GSI}{11}}} \right)$$

- Pappas and Mark (1993) show that the modulus of rock drops in a non-linear fashion with increased bulking, and that the rate of modulus change is a function of fragment shape and intact strength
- In *IMASS* the modulus is updated constantly via the zone-based volumetric strains. This allows for both modulus softening (during bulking) and modulus hardening (e.g., during recompaction)



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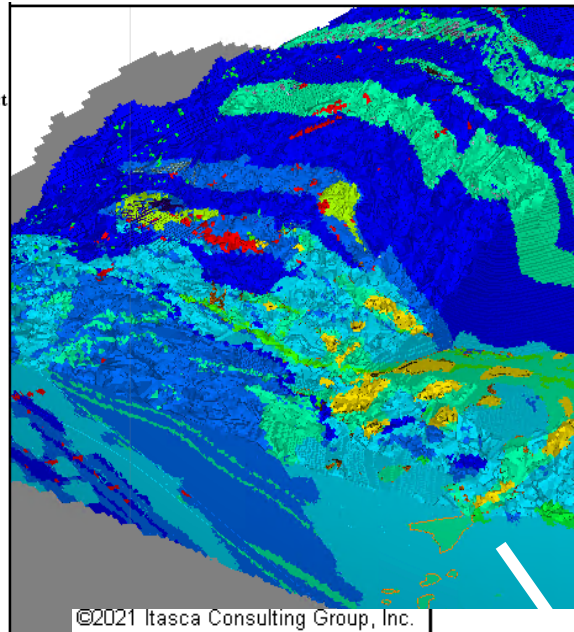
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Zone Property in_mod_youngintact

5.0000E+10
4.5000E+10
4.0000E+10
3.5000E+10
3.0000E+10
2.5000E+10
2.0000E+10
1.5000E+10
1.0000E+10
5.0000E+09
0.0000E+00

Intact rock
Young's
modulus



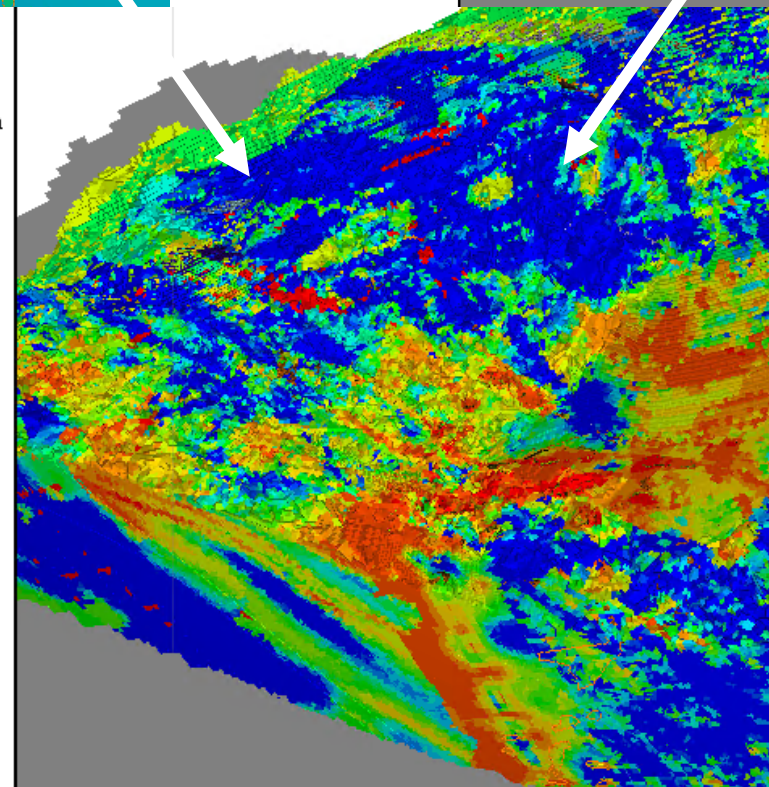
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Mechanical step : 253958

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Zone Property calc_mod_youngrm

1.5000E+10
1.4000E+10
1.3000E+10
1.2000E+10
1.1000E+10
1.0000E+10
9.0000E+09
8.0000E+09
7.0000E+09
6.0000E+09
5.0000E+09
4.0000E+09
3.0000E+09
2.0000E+09
1.0000E+09
0.0000E+00



Rock
mass
modulus

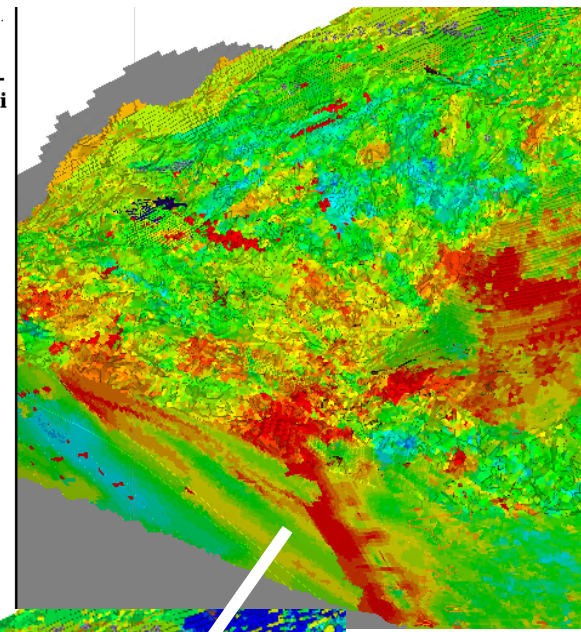
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Zone Property in_stren_gsi

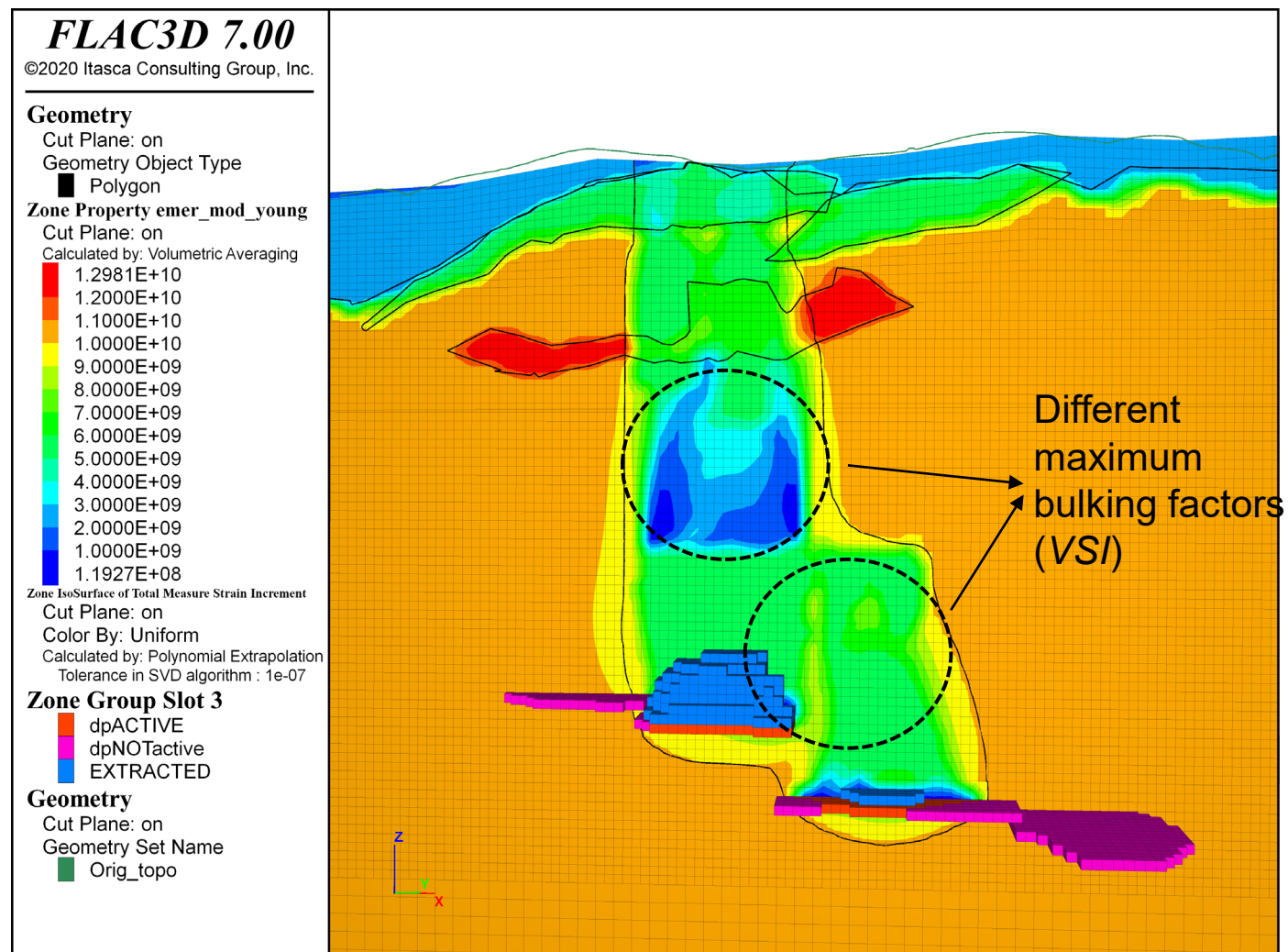
1.0000E+02
9.5000E+01
9.0000E+01
8.5000E+01
8.0000E+01
7.5000E+01
7.0000E+01
6.5000E+01
6.0000E+01
5.5000E+01
5.0000E+01
4.5000E+01
4.0000E+01
3.5000E+01
3.0000E+01
2.5000E+01
2.0000E+01
1.5000E+01



GSI



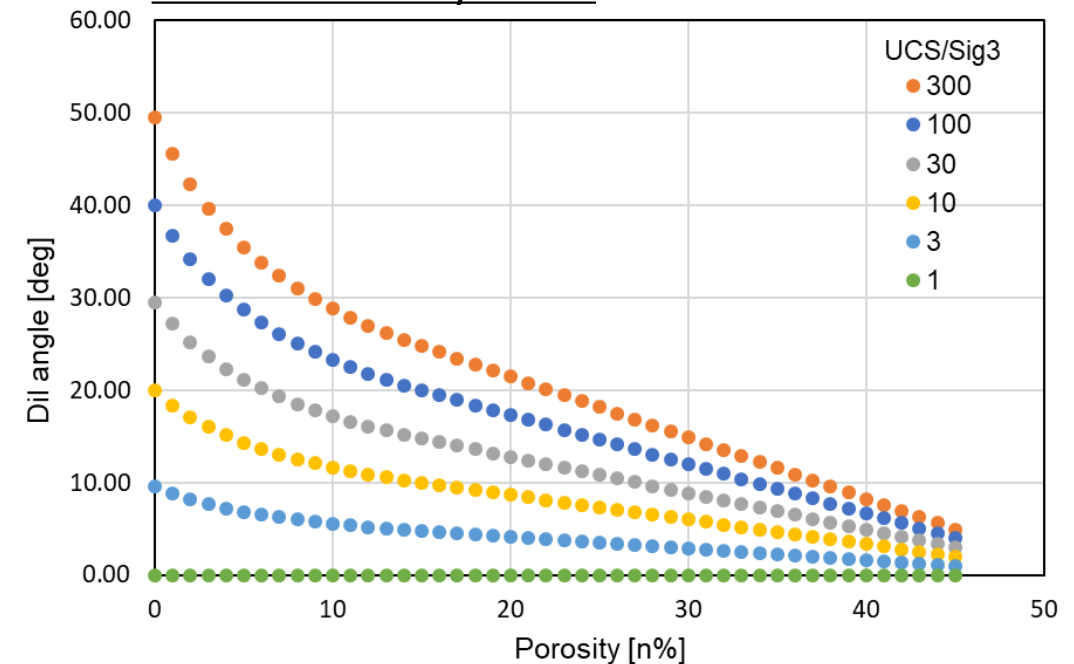
Modulus softening



Dilational behavior

- Within IMASS, the dilation angle is set as a standard material property and drops to zero once the user-defined maximum bulking factor is reached. This prevents zones from expanding to unrealistic levels during shear.
- A constant dilation angle can be assigned to the rock mass (i.e., equal everywhere) based on the available guidelines such as those provided by Hoek and Brown (1997).
- Alternatively, a more advanced dilation model is available that constantly updates the dilation angle for each zone as a function of confining stress and porosity (or VSI).

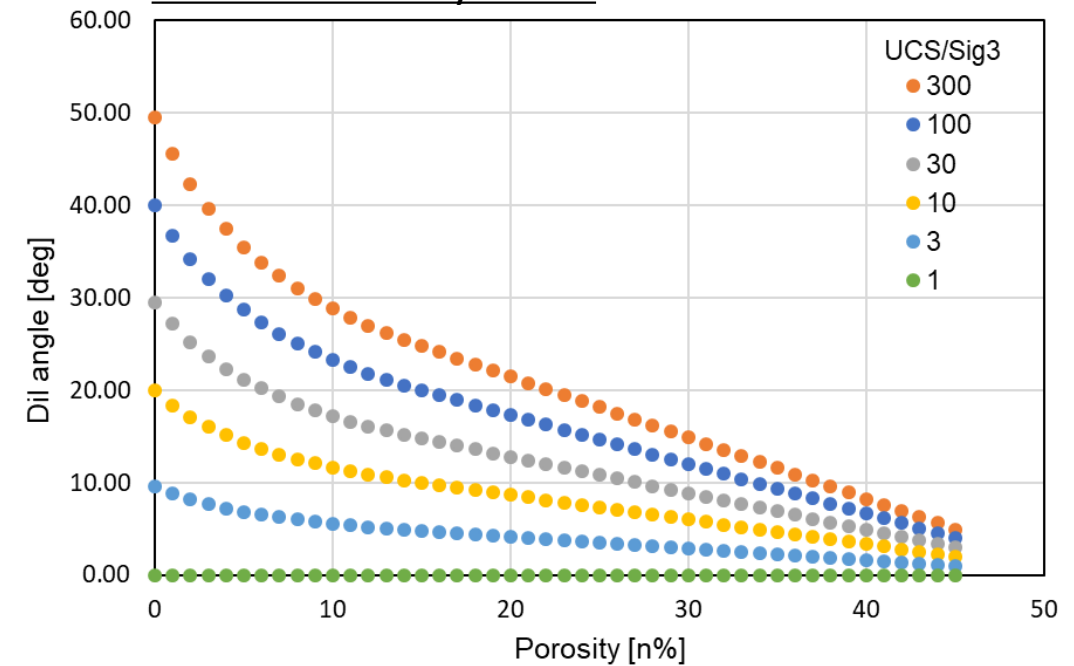
Currently being tested
and validated by Itasca



Dilational behavior

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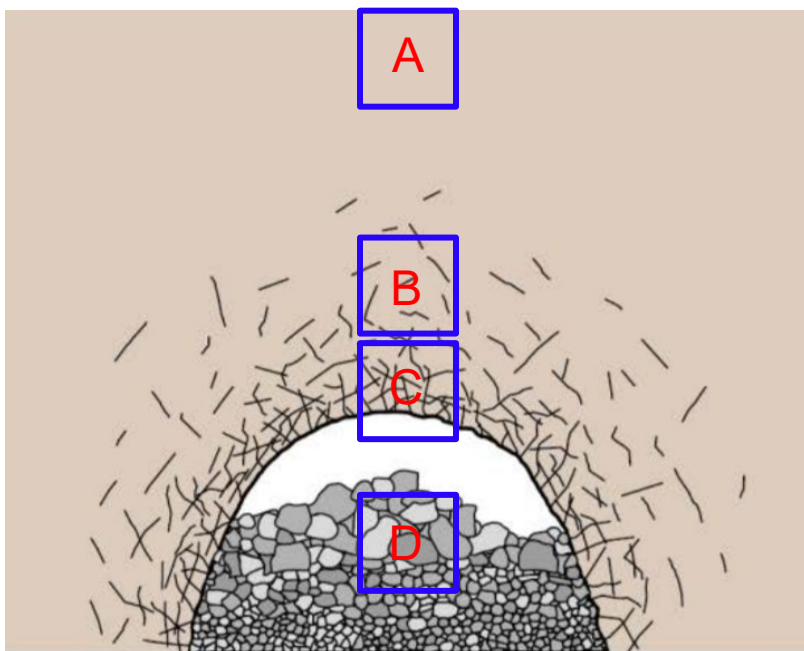
Currently being tested
and validated by Itasca



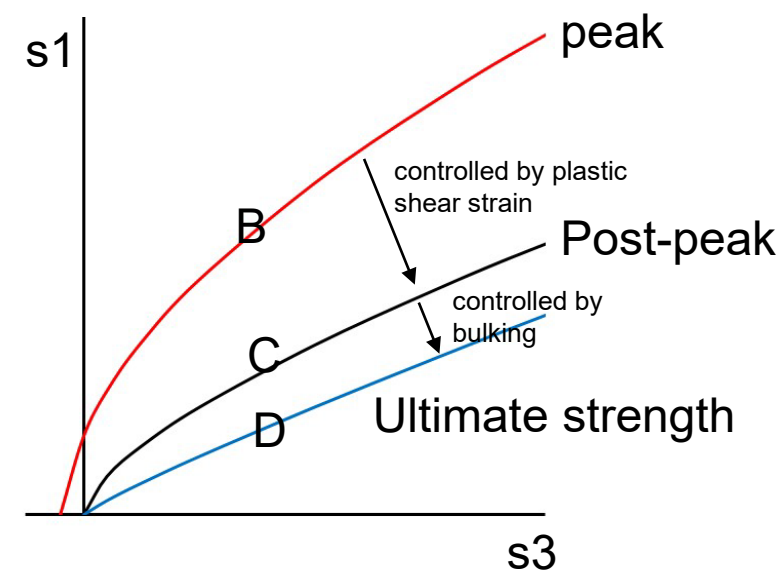
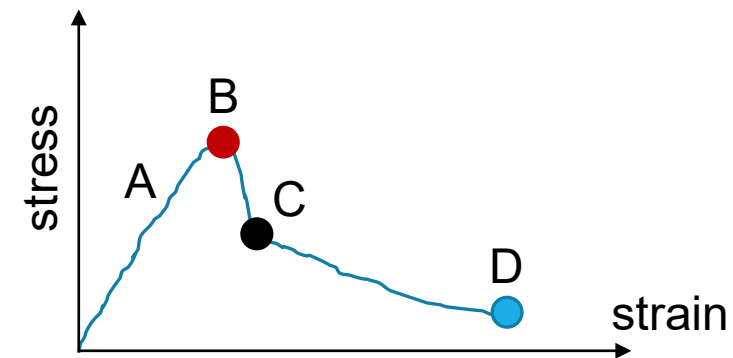
$$\tau = \sigma_n \tan \left(R \cdot \log \left(\frac{S}{\sigma_n} \right) + \phi_b \right)$$



sloss – an indicator for damage in *IMASS*



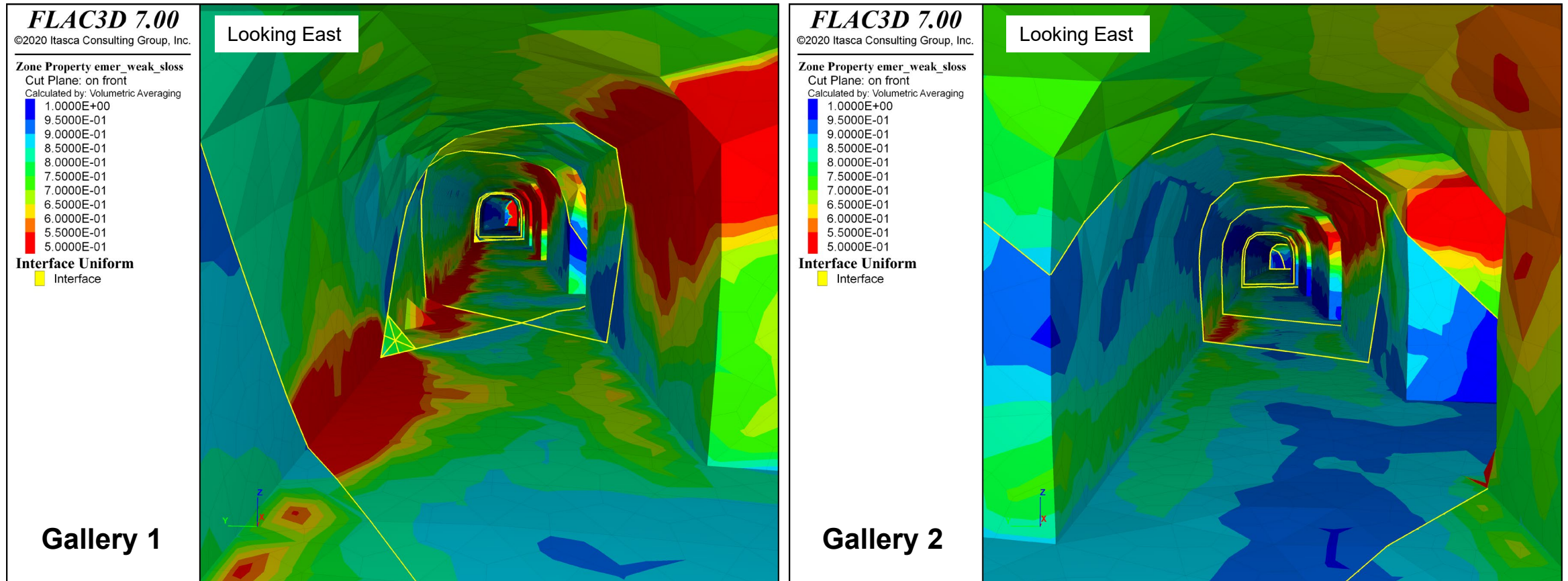
		cohesion	<i>sloss</i>	bulking
A		max	1	0
B			1	0
C		0	0	0
D		0	-1	max



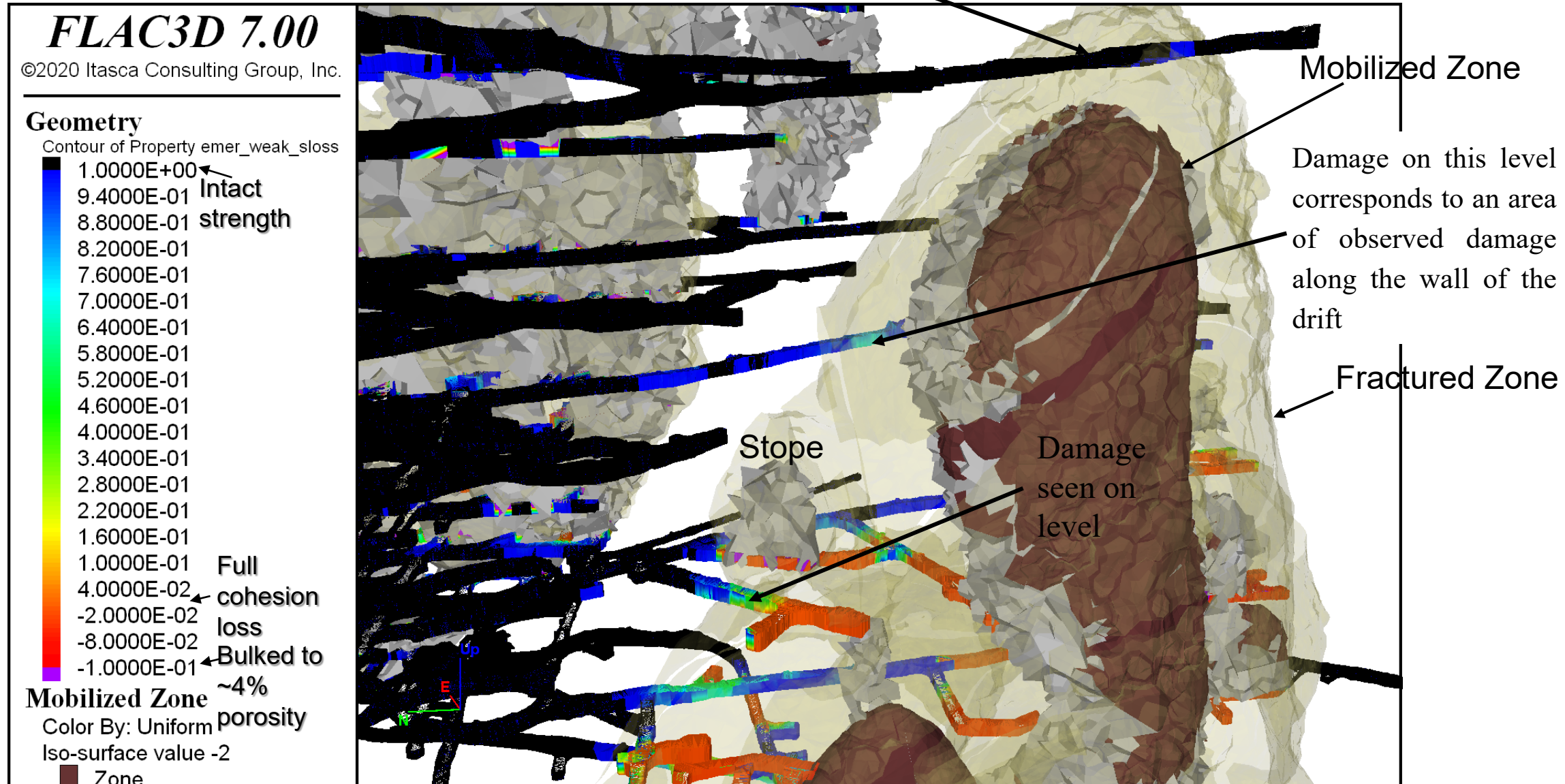
sloss changes between [1,-1]:

- Between Peak and post-peak envelope,
 $sloss = 1 - (\text{plastic shear strain} / \text{critical plastic shear strain})$
- Between post-peak and ultimate strength envelope
 $sloss = - (\text{volumetric strain} / \text{max allowable volumetric strain})$

sloss example



No signs of damage or fracturing in most of the level – in line with observations

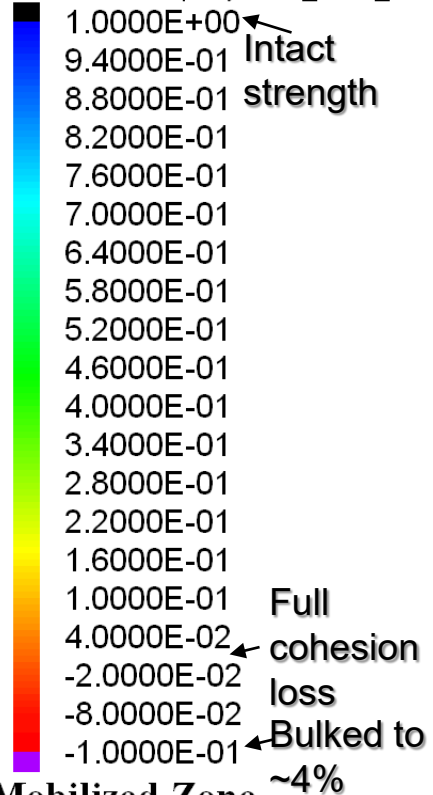


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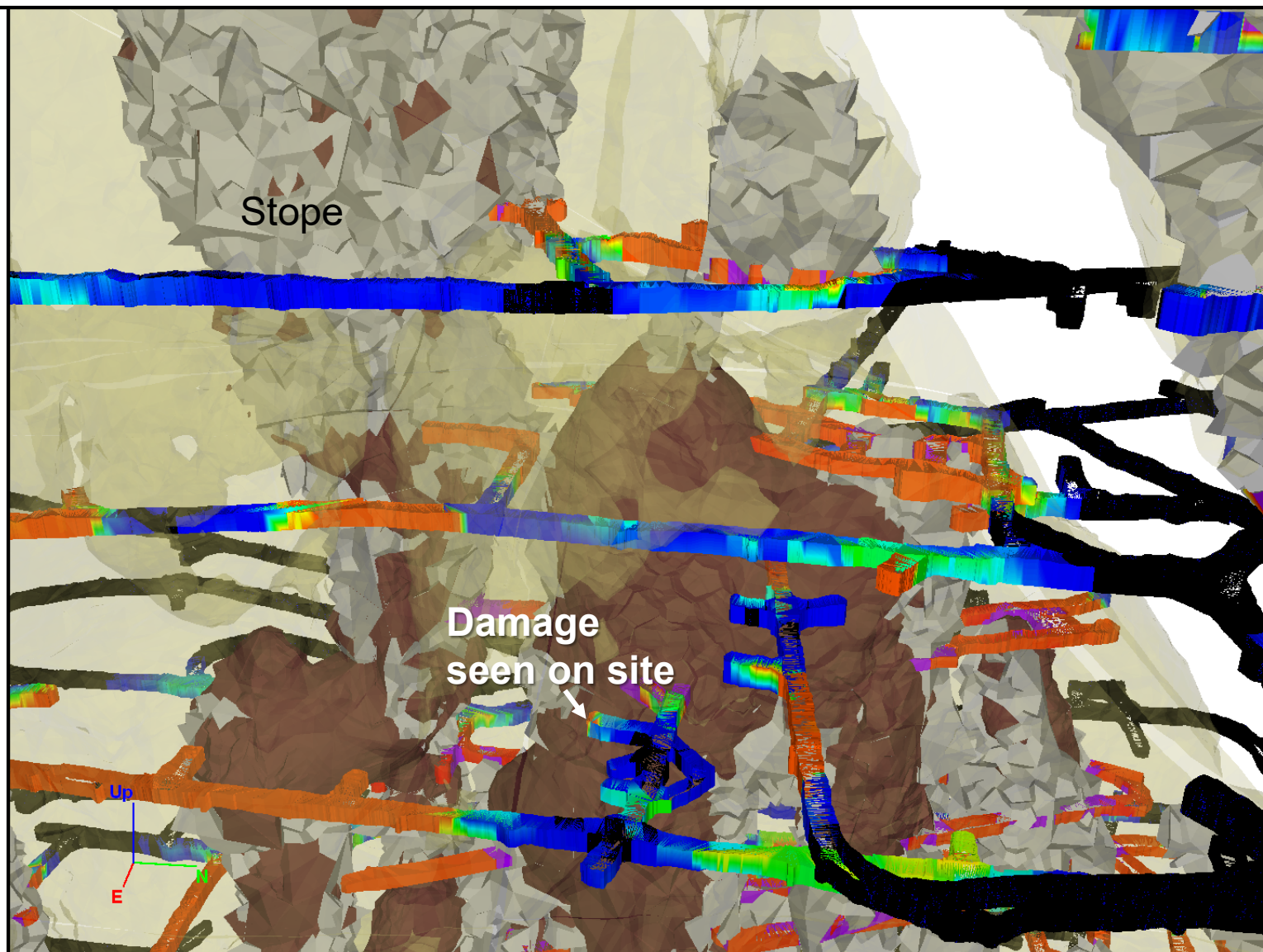
Contour of Property emer_weak_sloss



Mobilized Zone

Color By: Uniform
Iso-surface value -2

Zone



Current Conditions



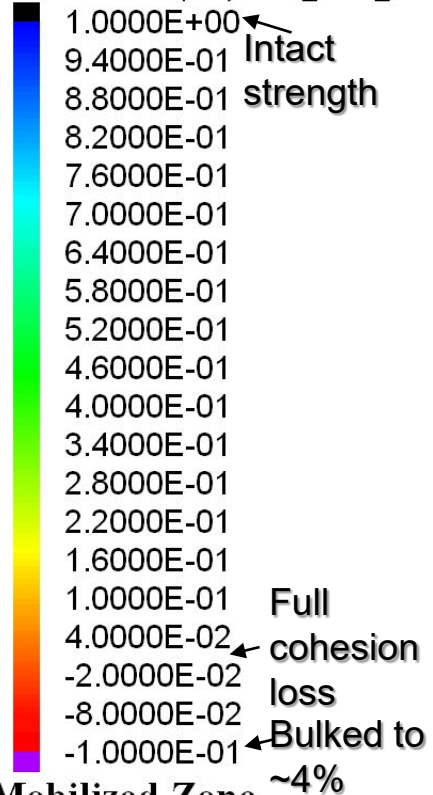
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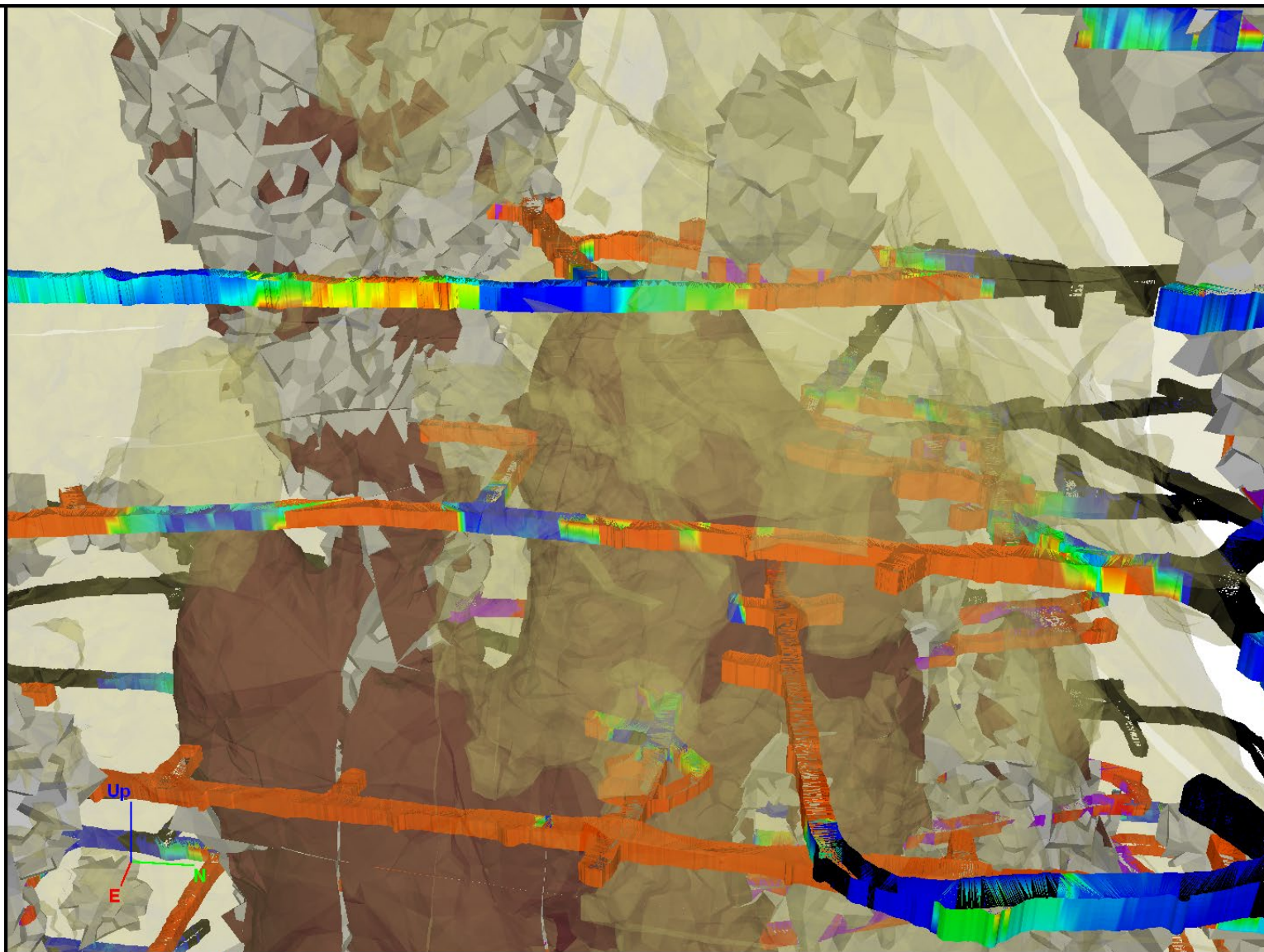
Contour of Property emer_weak_sloss



Mobilized Zone

Color By: Uniform
Iso-surface value -2

Zone



Year 3



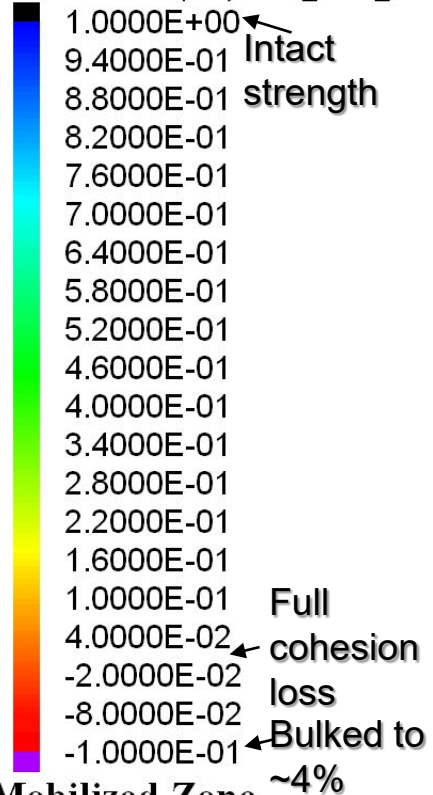
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Contour of Property emer_weak_sloss

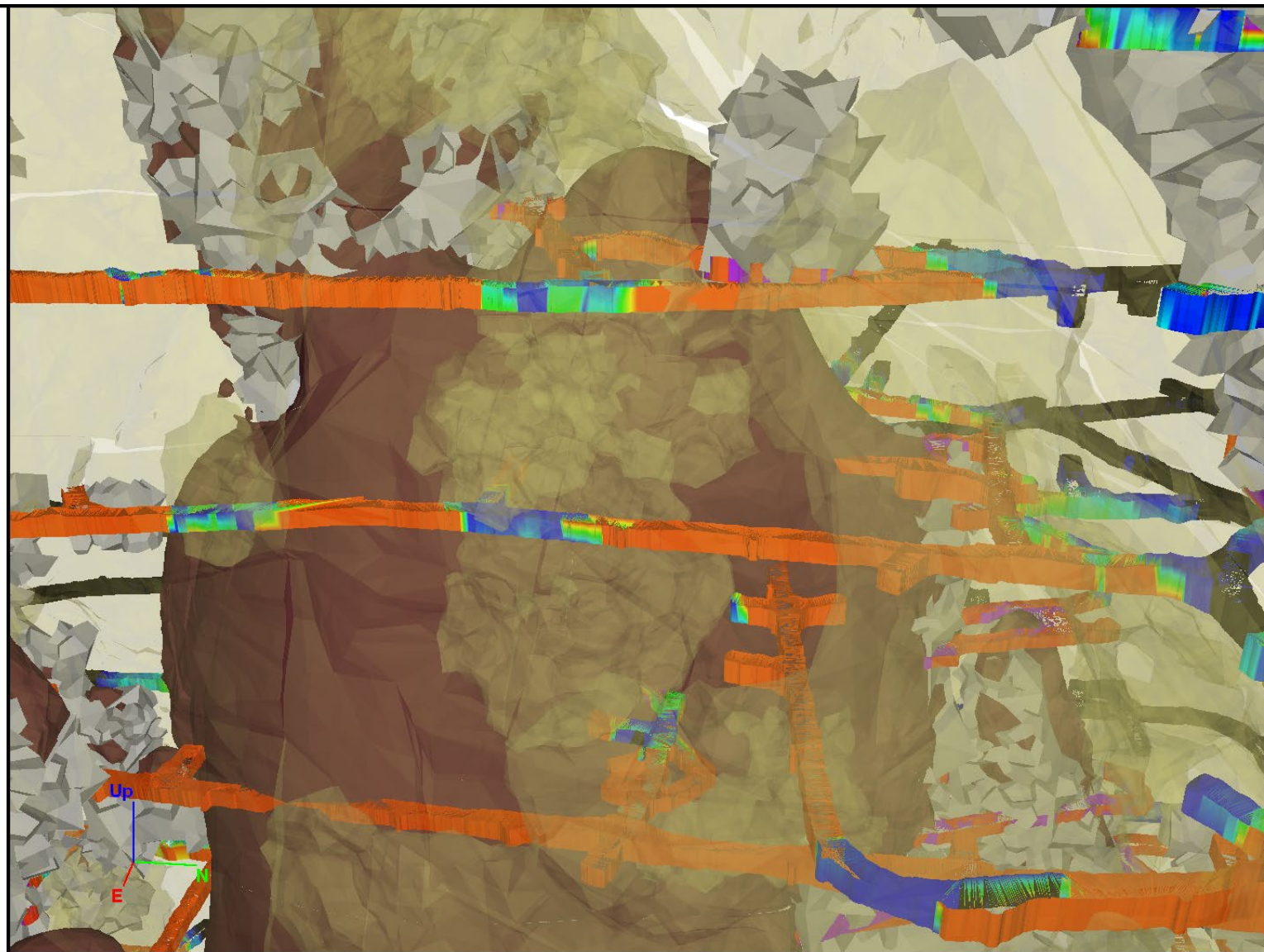


Mobilized Zone

Color By: Uniform

Iso-surface value -2

Zone



Year 6



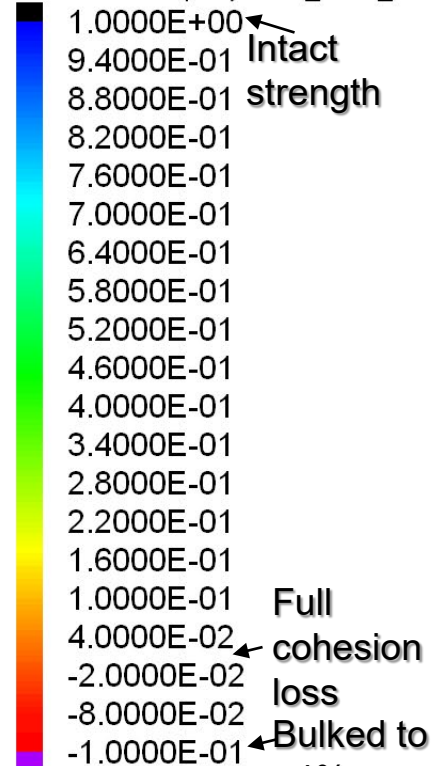
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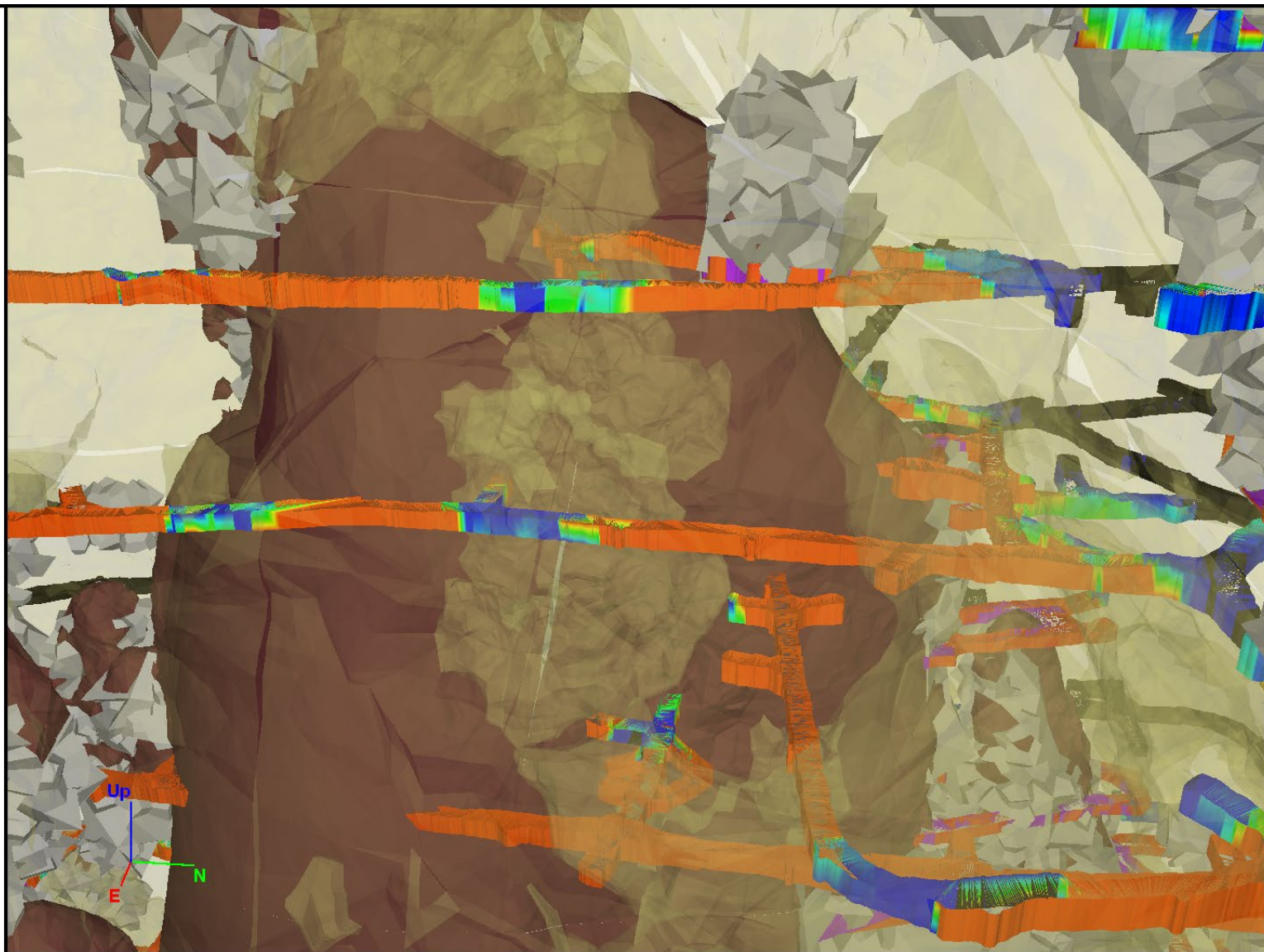
Contour of Property emer_weak_sloss



Mobilized Zone

Color By: Uniform
Iso-surface value -2

Zone



Year 7



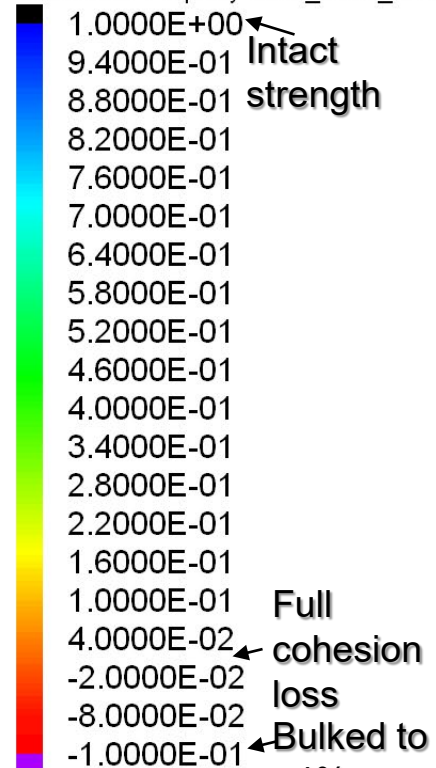
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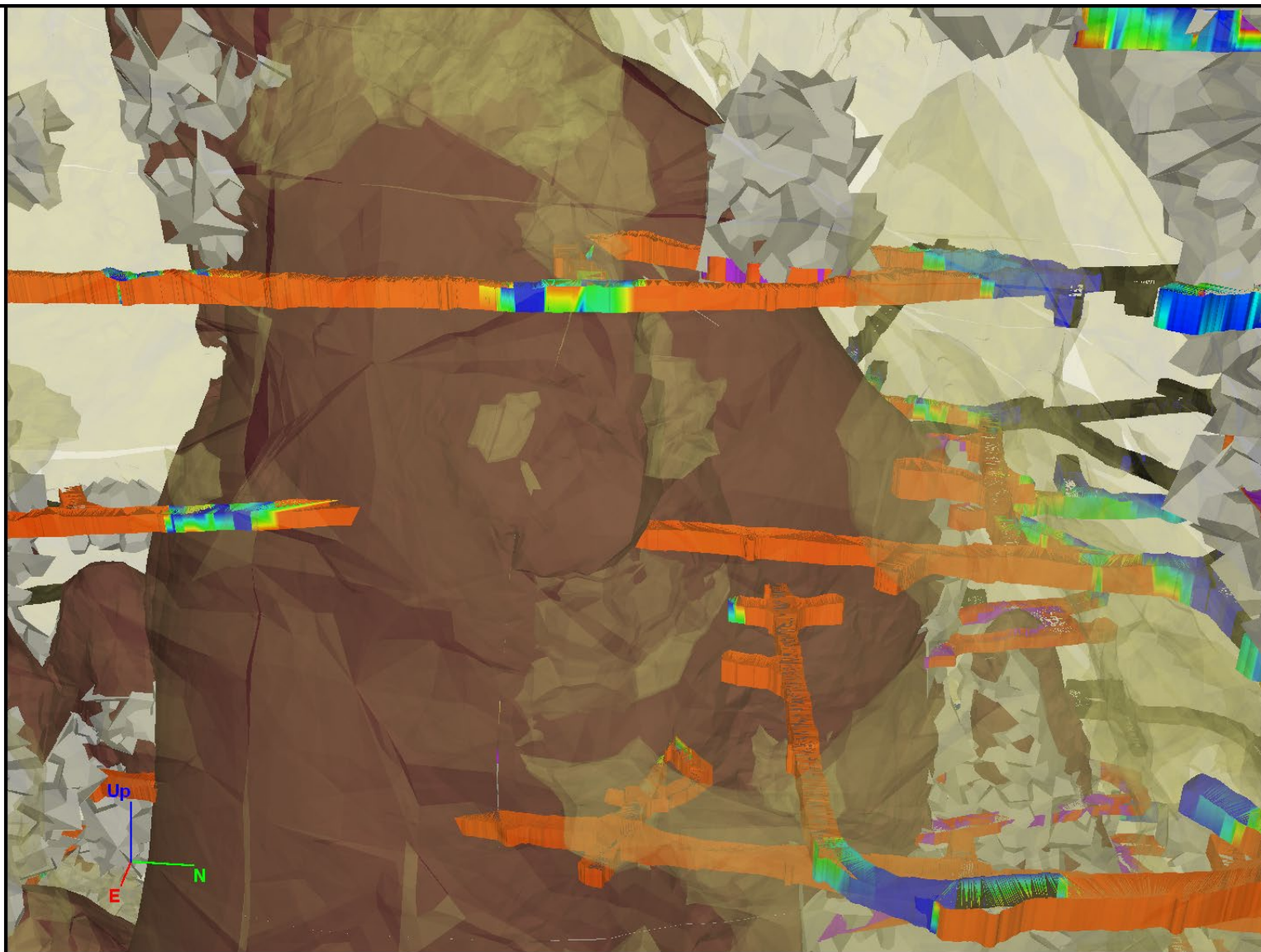
Contour of Property emer_weak_sloss



Mobilized Zone

Color By: Uniform
Iso-surface value -2

Zone



Year 8



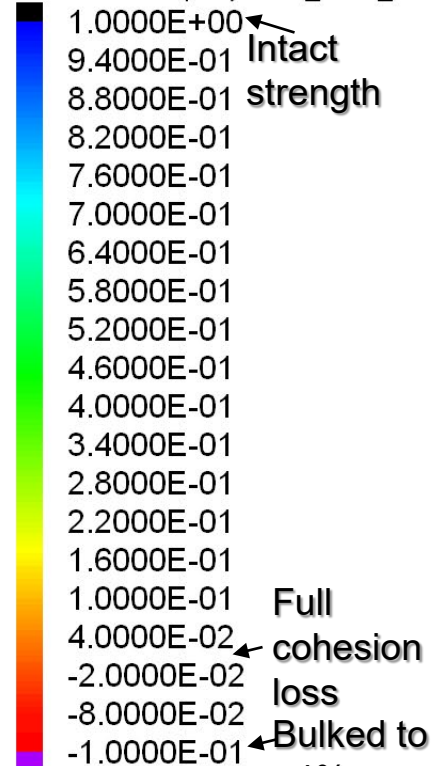
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Contour of Property emer_weak_sloss

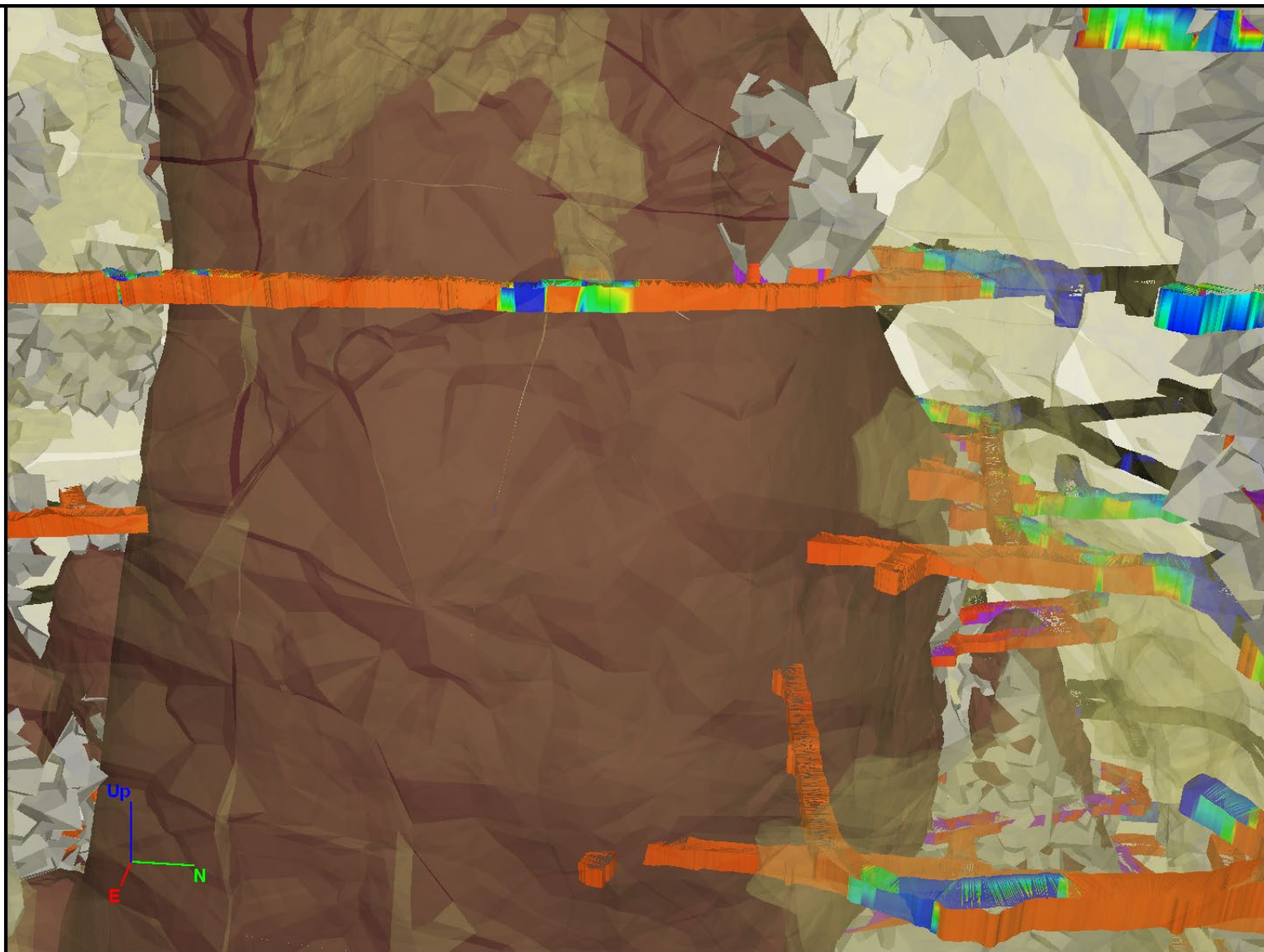


Mobilized Zone

Color By: Uniform porosity

Iso-surface value -2

Zone



Year 9



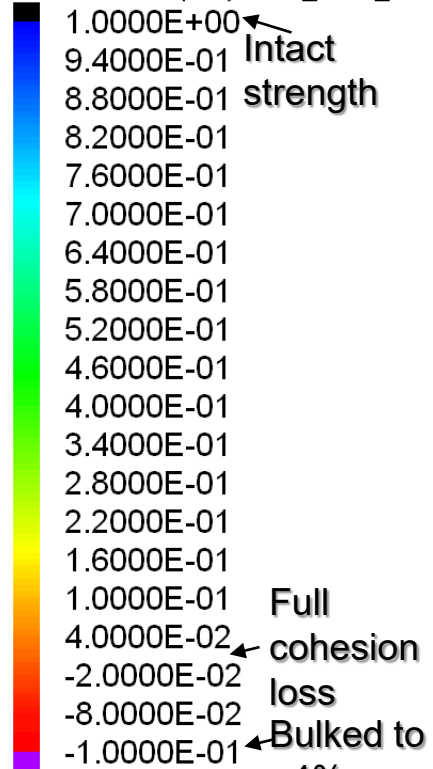
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Contour of Property emer_weak_sloss

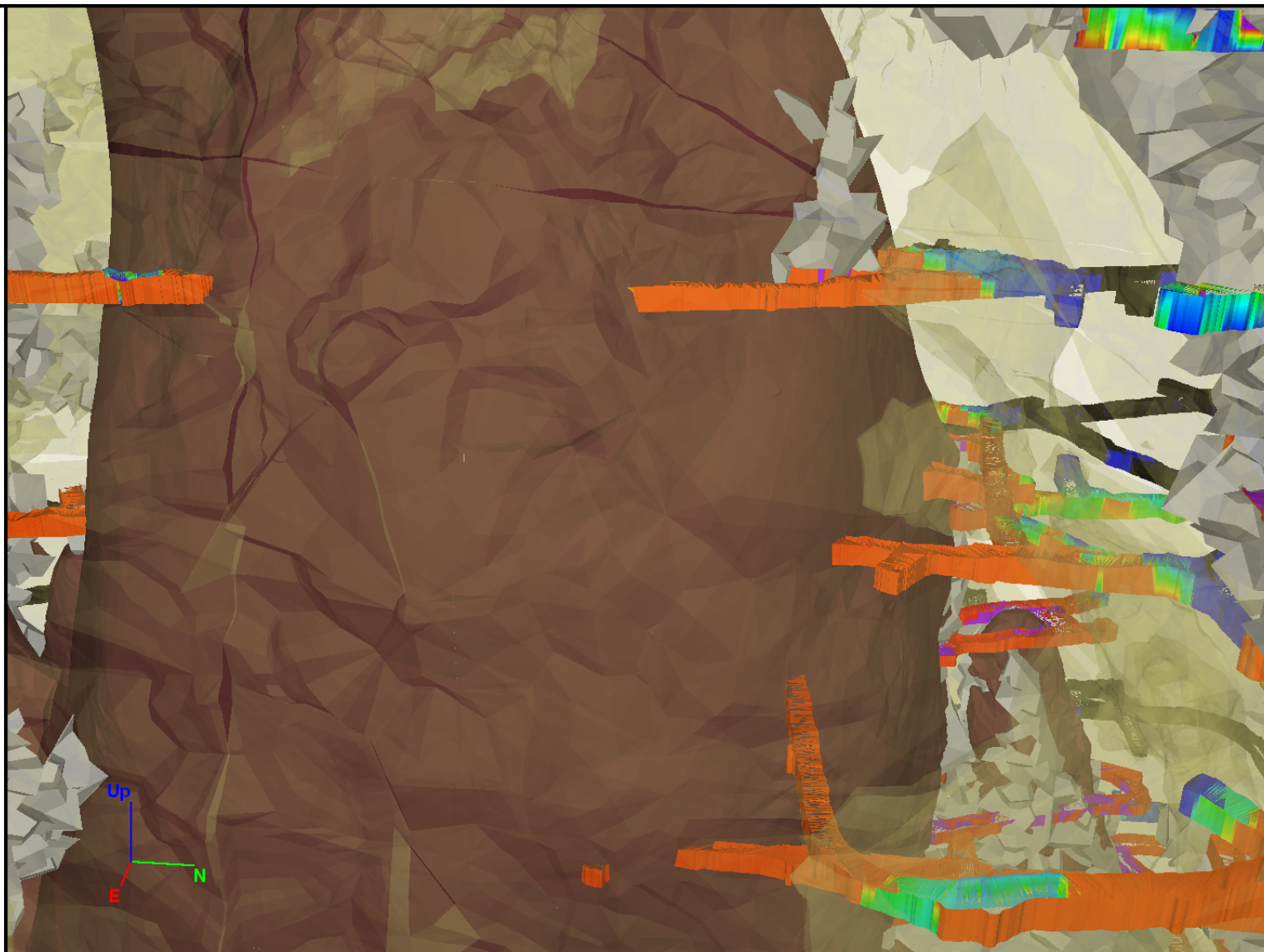


Mobilized Zone

Color By: Uniform porosity

Iso-surface value -2

Zone



Year 10



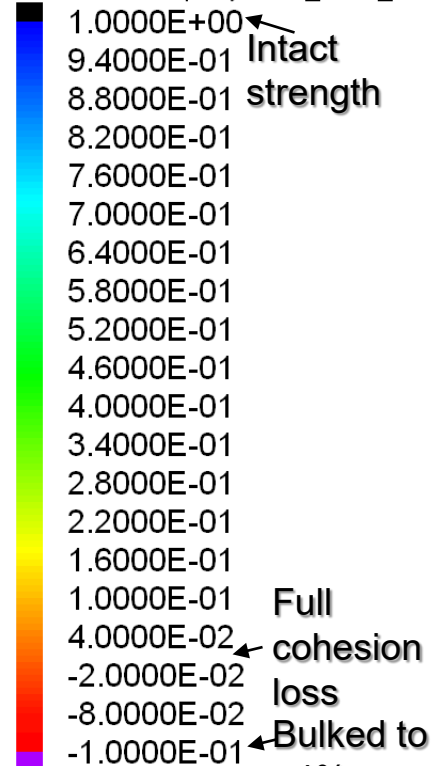
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Geometry

Contour of Property emer_weak_sloss



Mobilized Zone

Color By: Uniform porosity

Iso-surface value -2

Zone



Year 11



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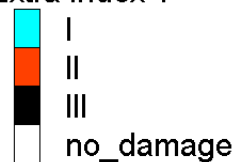
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Extra Index 1



Zone IsoSurface of Total Measure Strain Increment

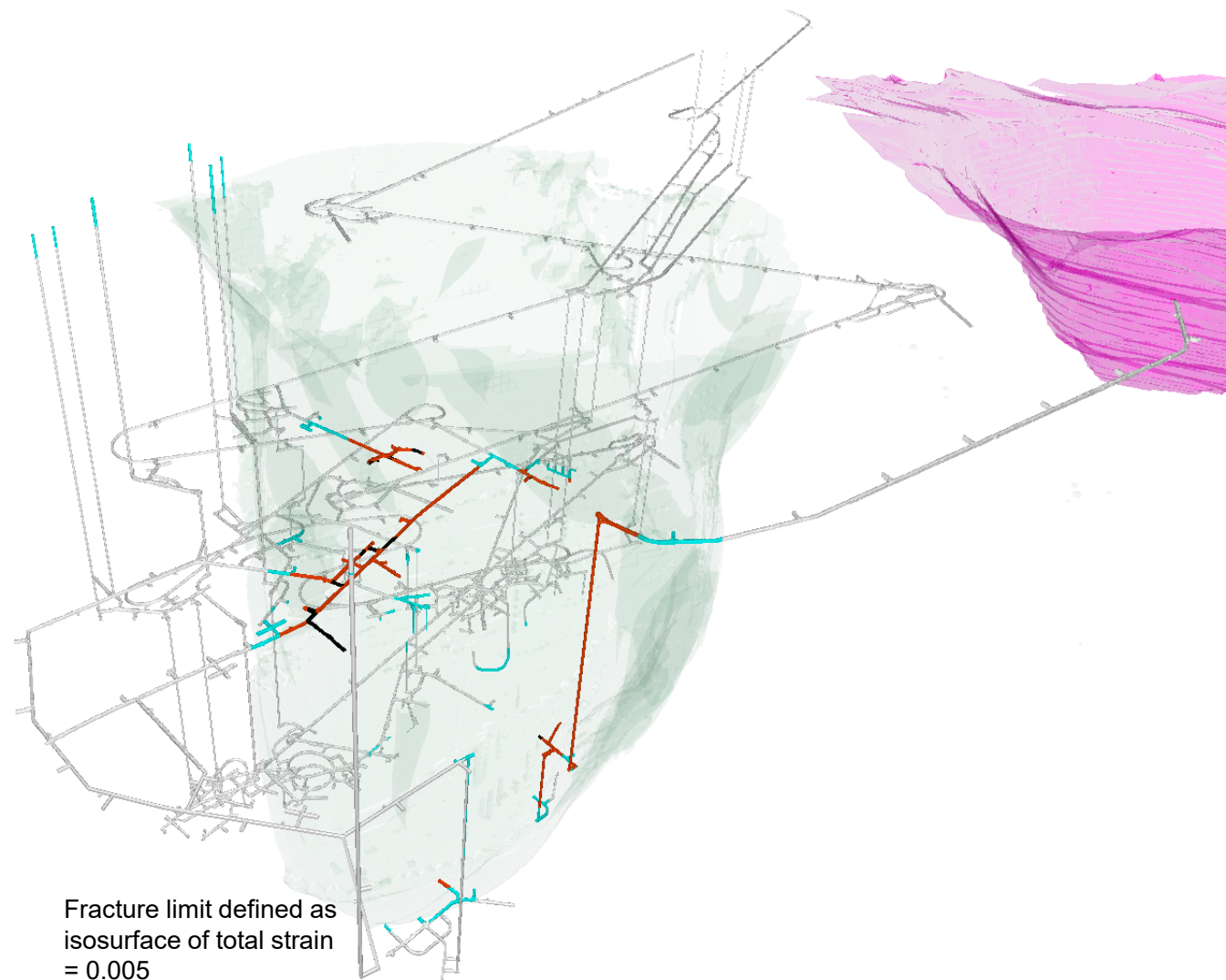
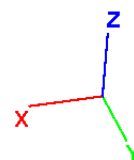
Color By: Zone Applied Conditions

Iso-surface value 0.005

 None

Calculated by: Polynomial Extrapolation

Tolerance in SVD algorithm : 1e-07



UG_infra



ITASCA™

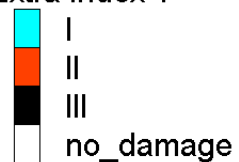
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Extra Index 1



Zone IsoSurface of Total Measure Strain Increment

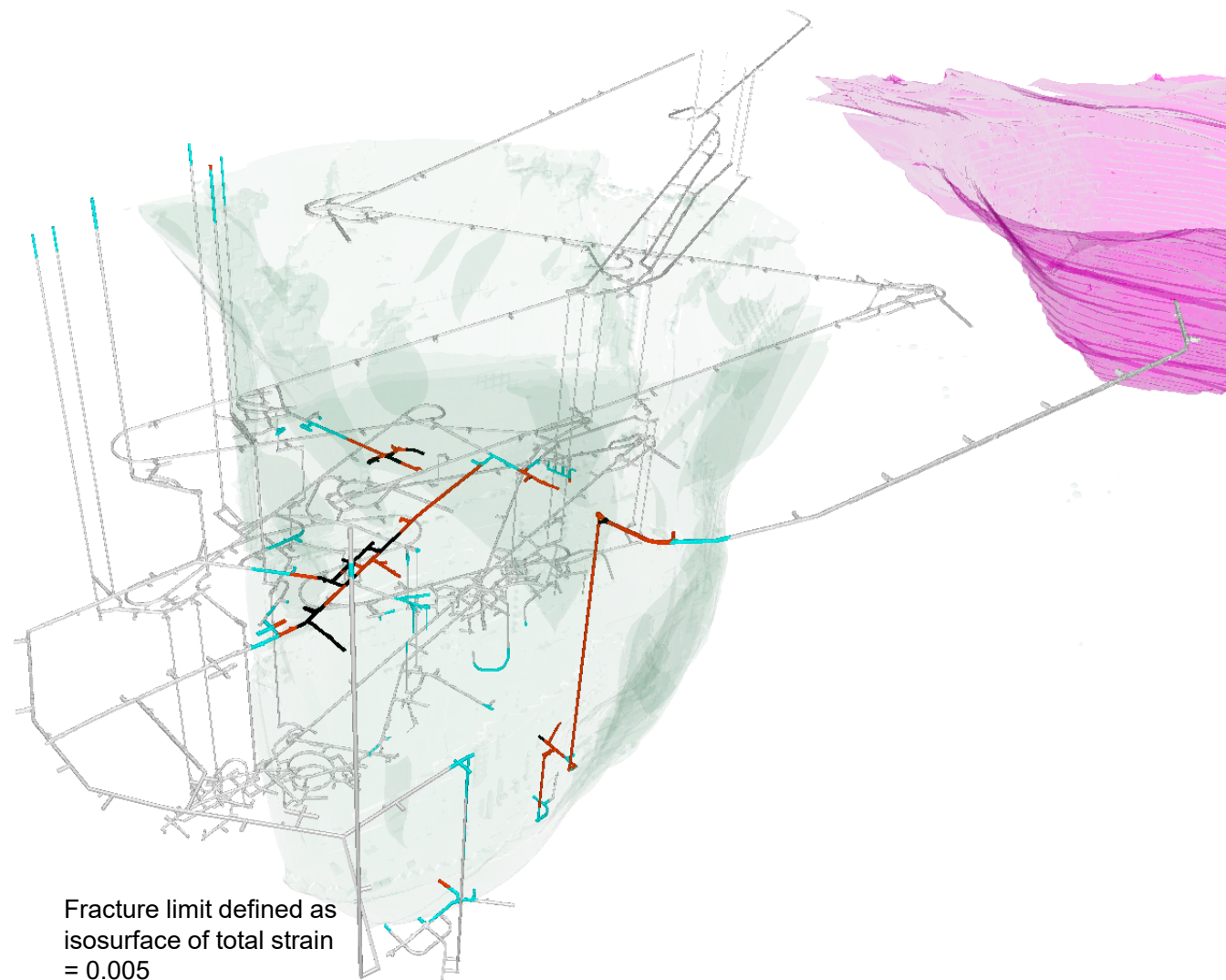
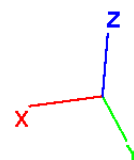
Color By: Zone Applied Conditions

Iso-surface value 0.005

 None

Calculated by: Polynomial Extrapolation

Tolerance in SVD algorithm : 1e-07



UG_infra



ITASCA™

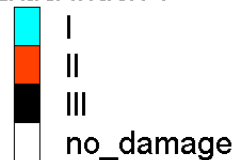
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Extra Index 1



Zone IsoSurface of Total Measure Strain Increment

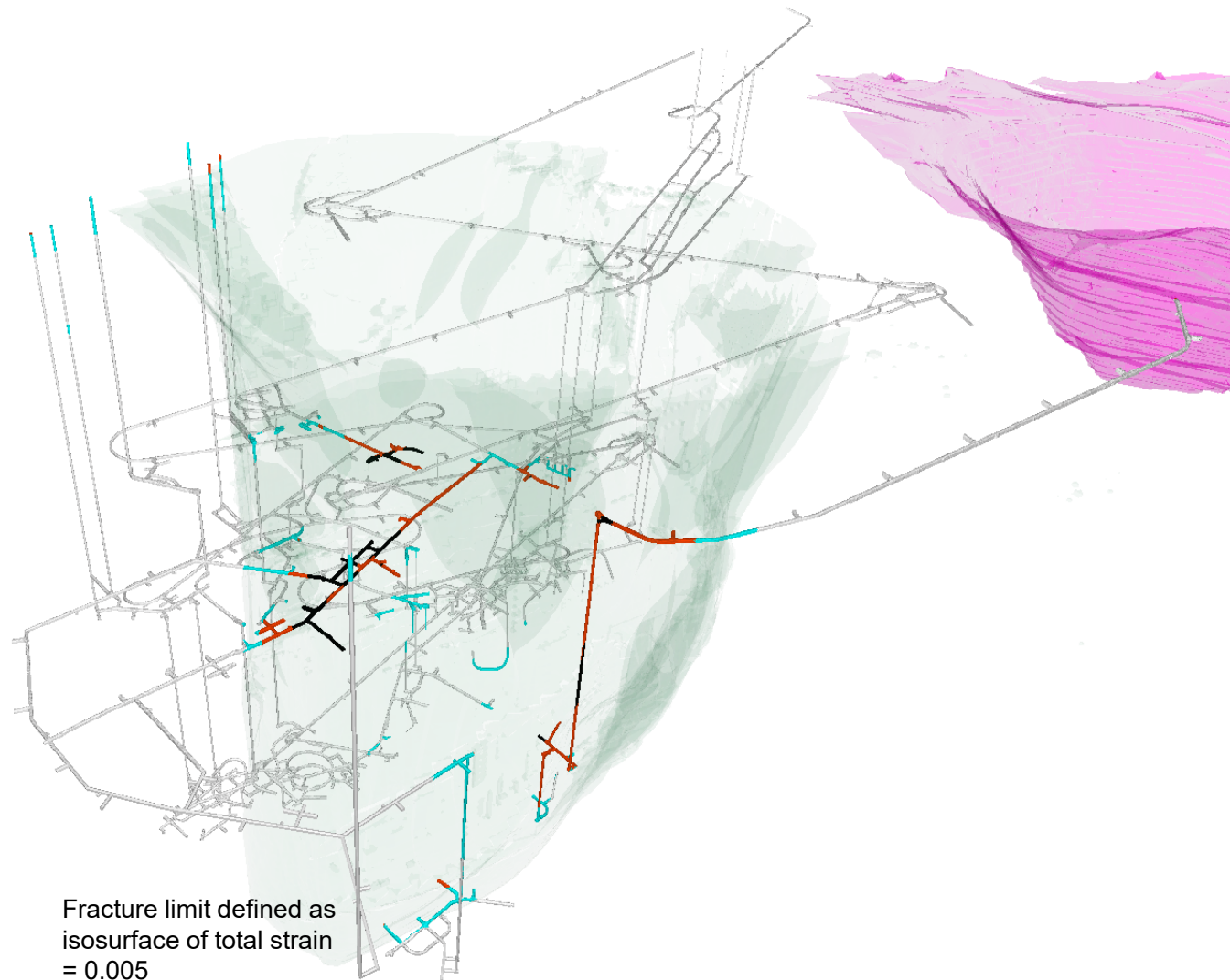
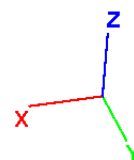
Color By: Zone Applied Conditions

Iso-surface value 0.005

 None

Calculated by: Polynomial Extrapolation

Tolerance in SVD algorithm : 1e-07



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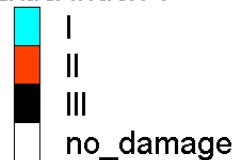
FLAC3D 7.00

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Geometry

Geometry

Extra Index 1



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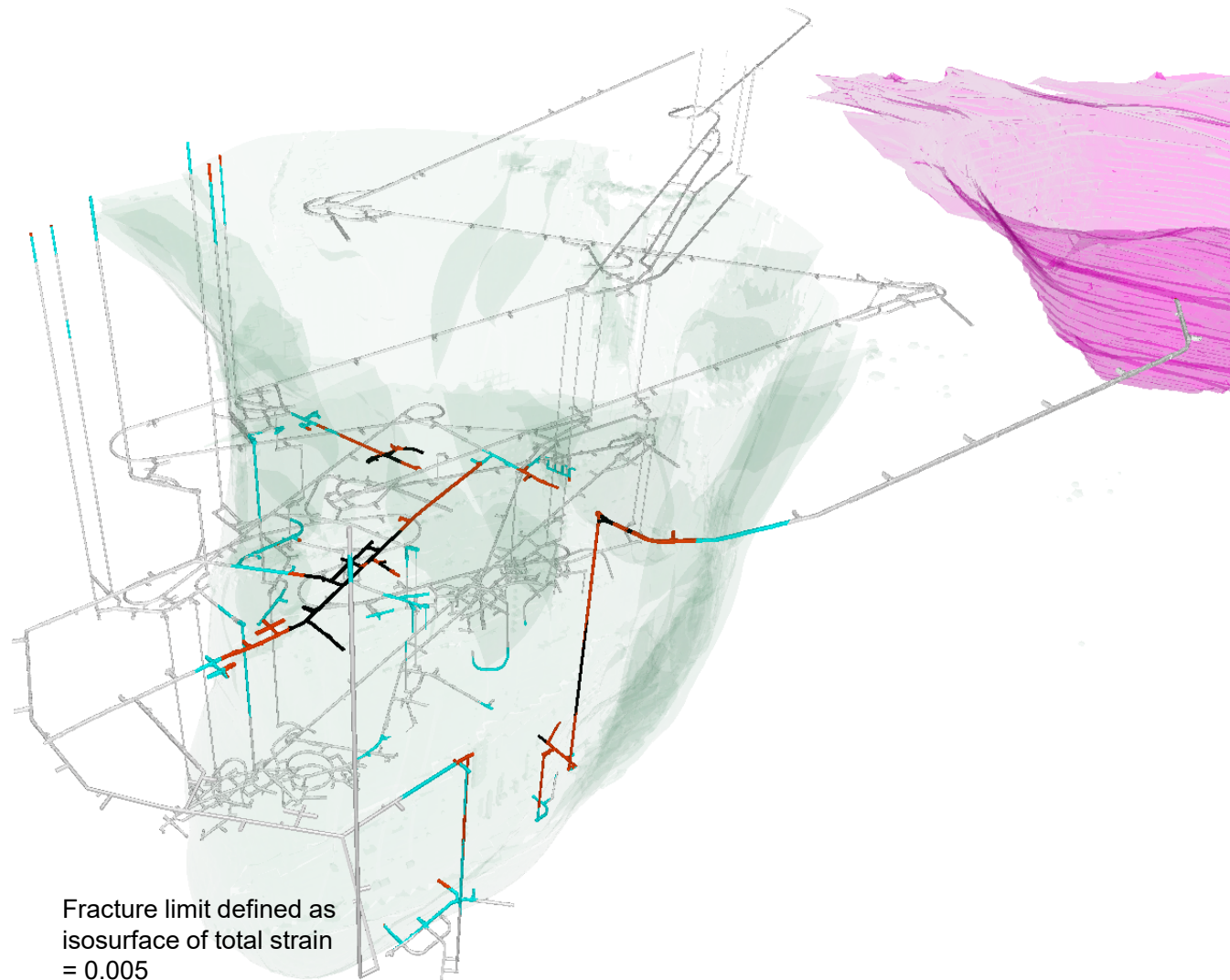
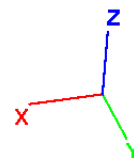
Color By: Zone Applied Conditions

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Fracture limit defined as
isosurface of total strain
= 0.005

UG_infra



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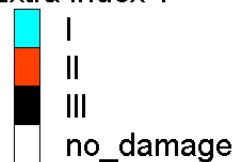
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Geometry

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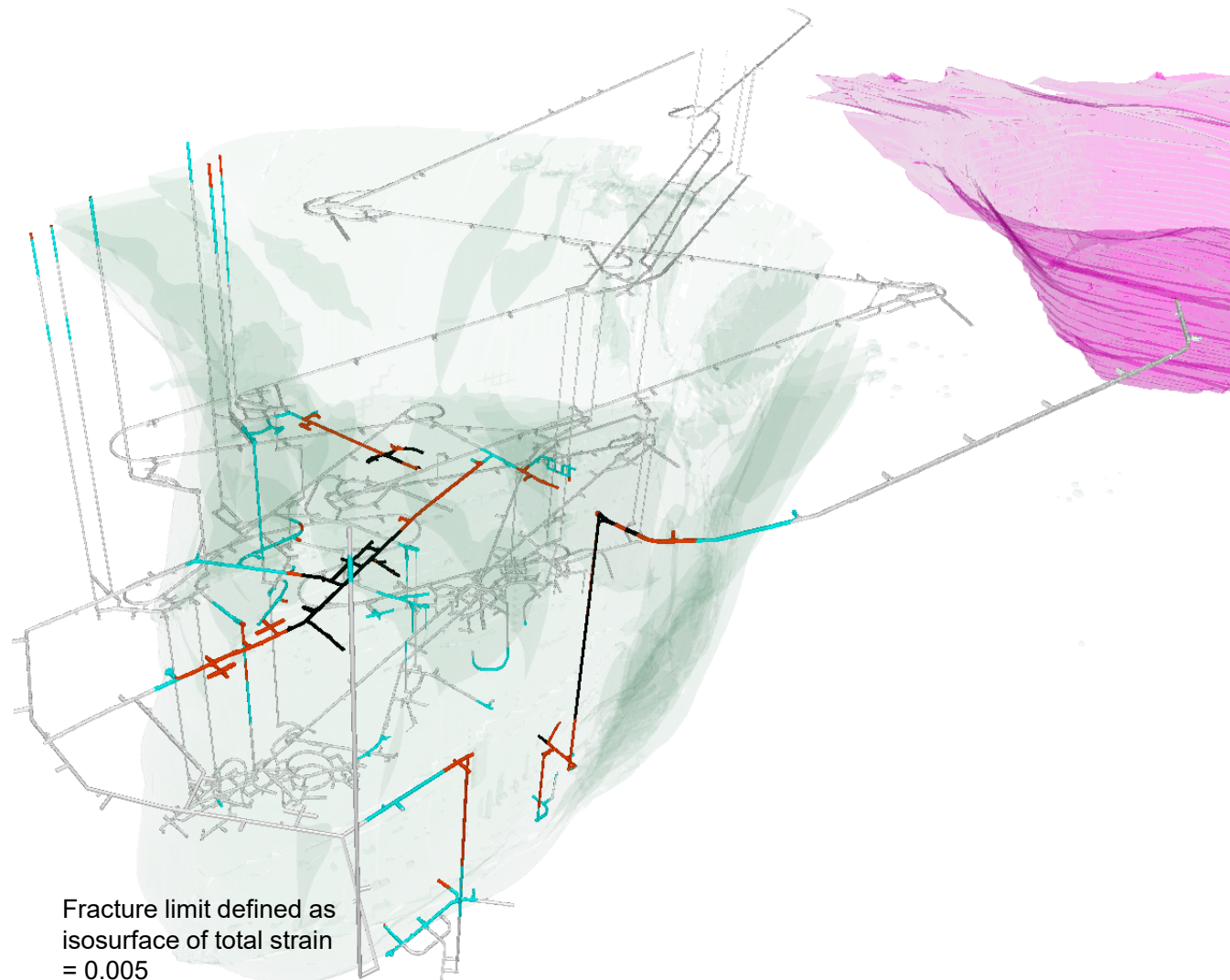
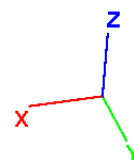
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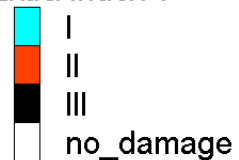
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Geometry

Geometry

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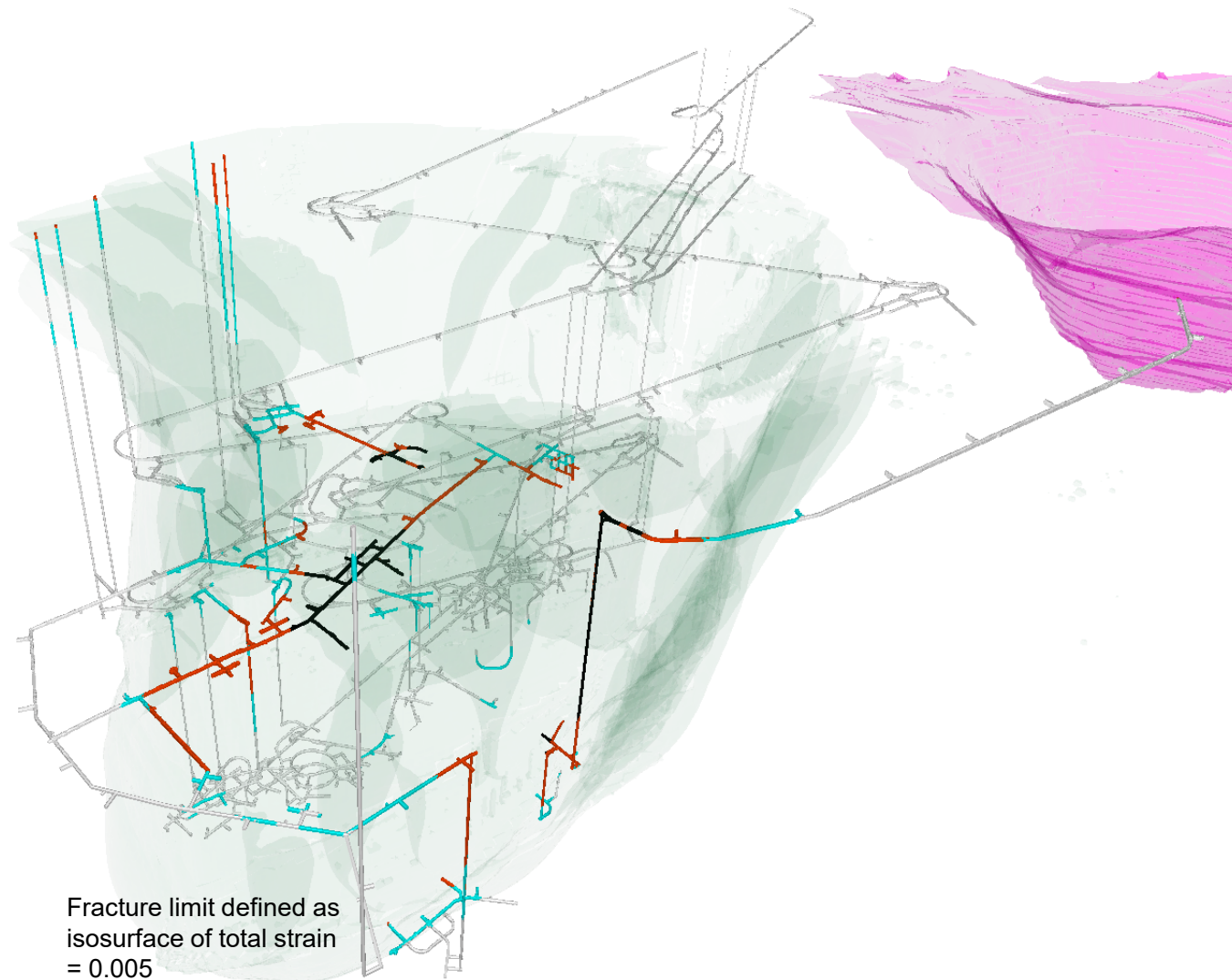
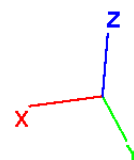
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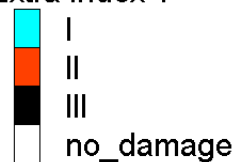
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Geometry

Geometry

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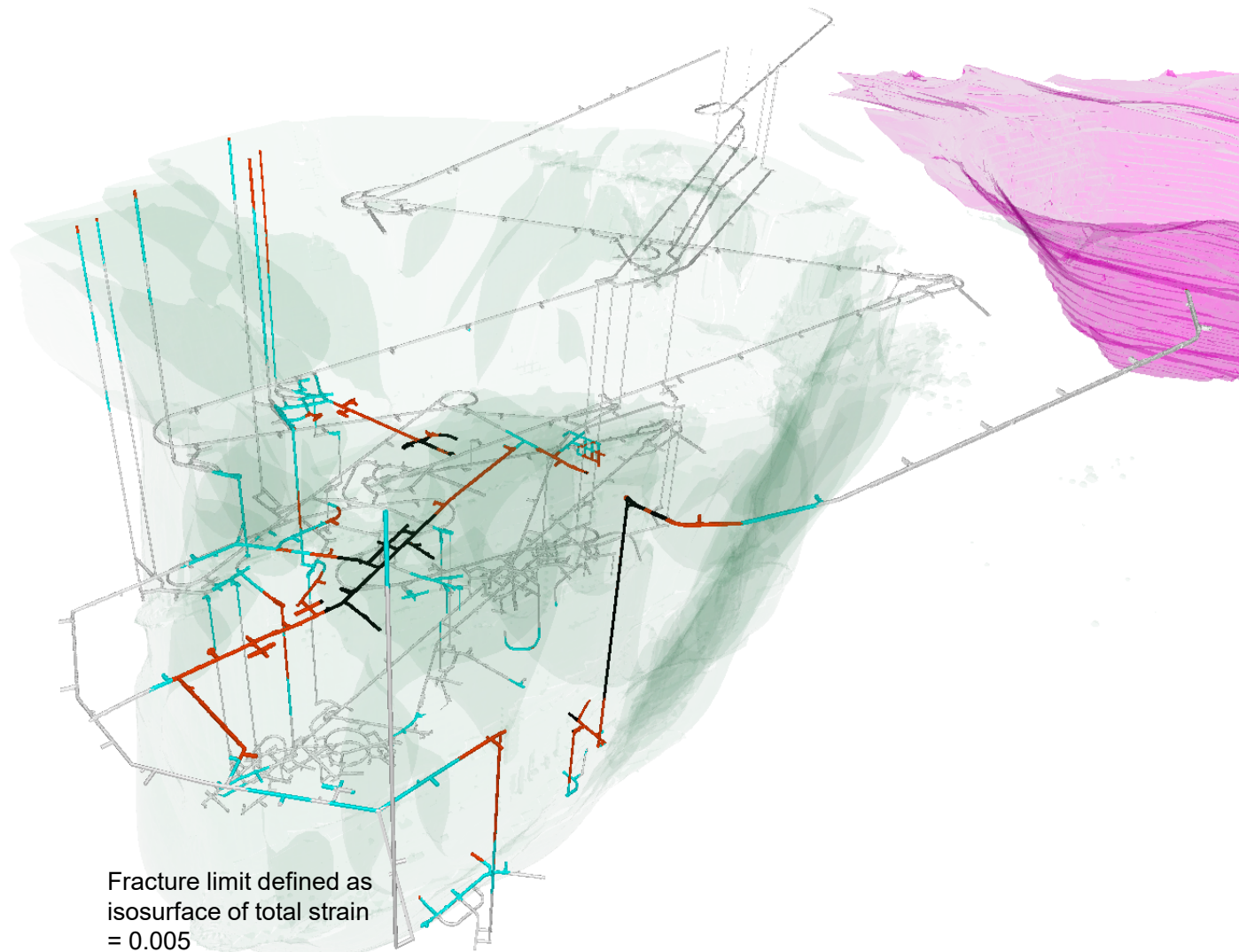
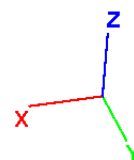
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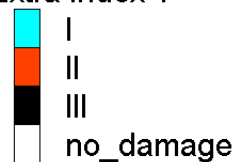
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Geometry

Geometry

Extra Index 1



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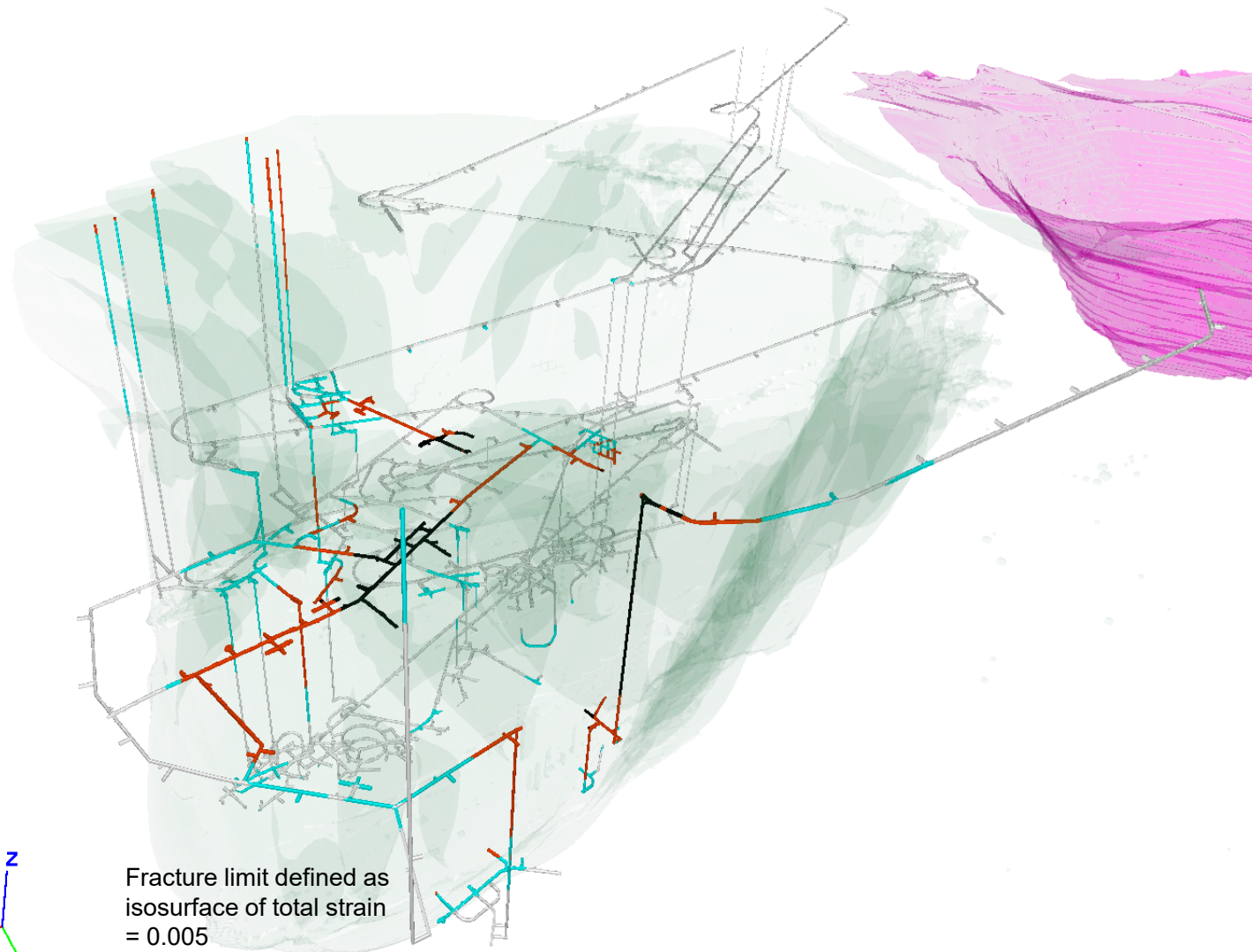
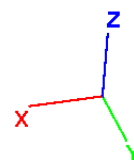
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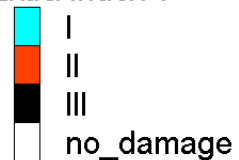
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Geometry

Geometry

Extra Index 1



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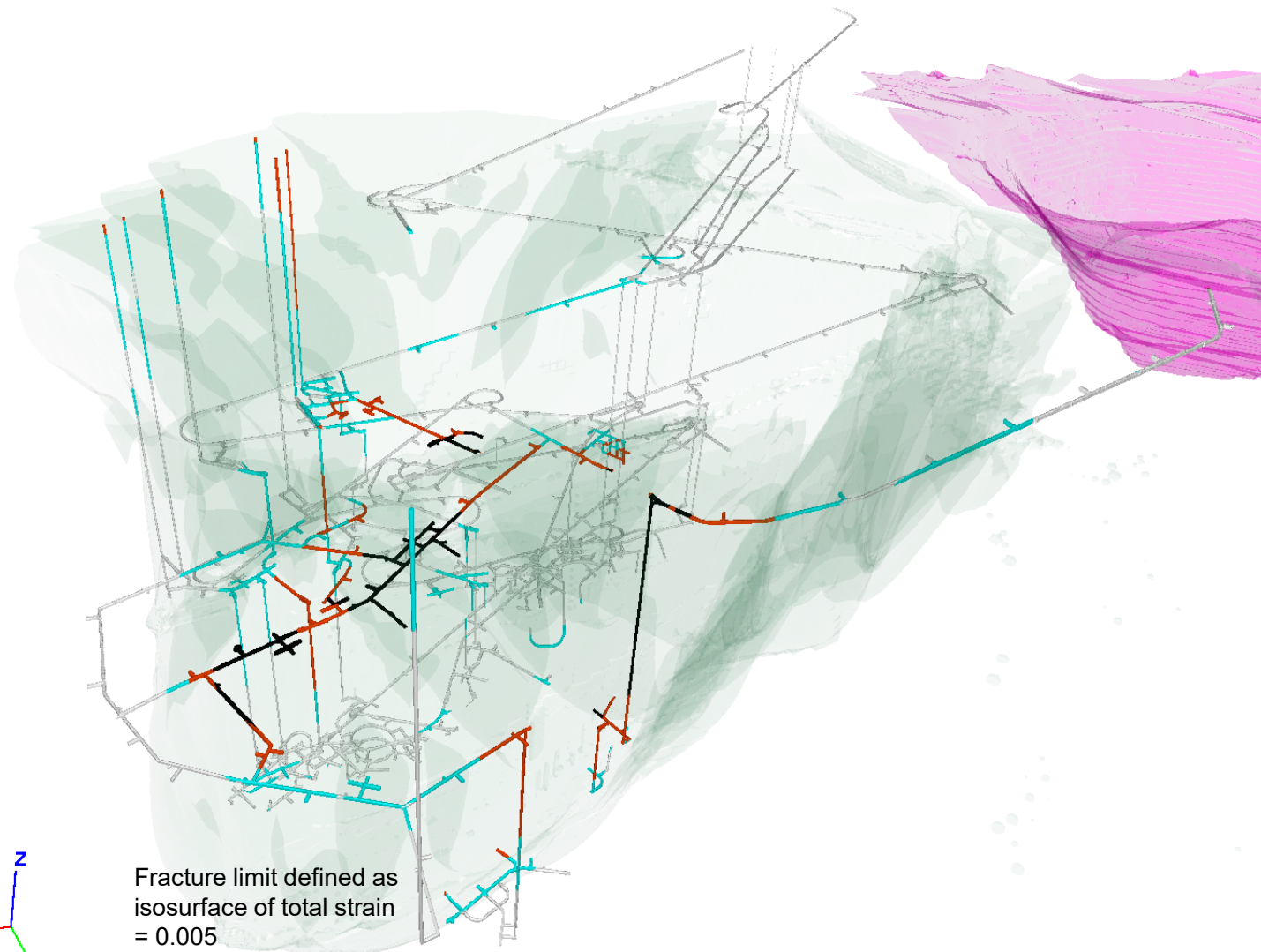
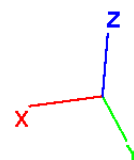
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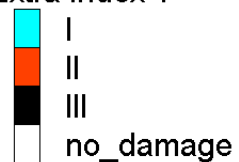
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Geometry

Geometry

Extra Index 1



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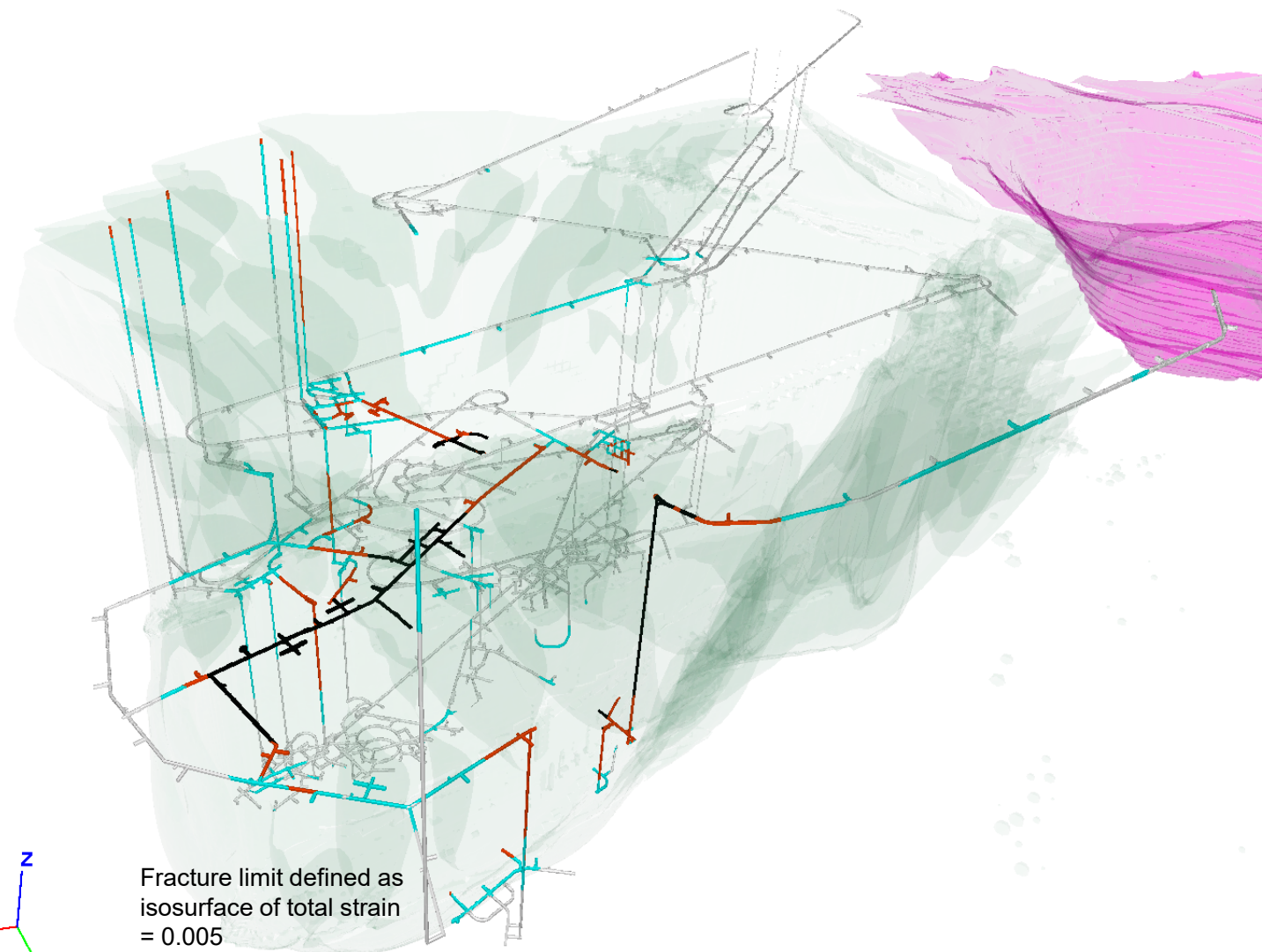
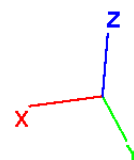
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UG_infra

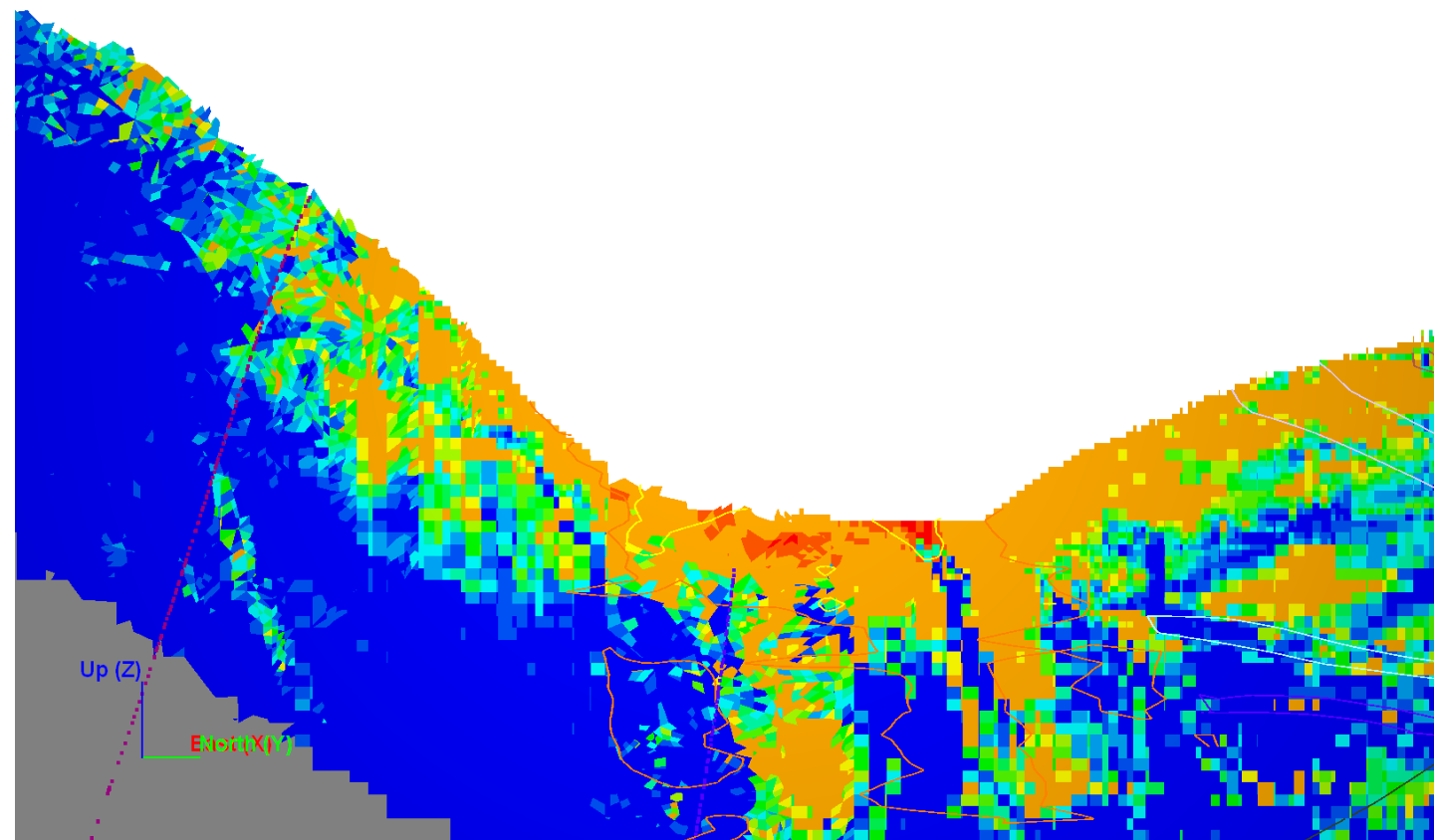
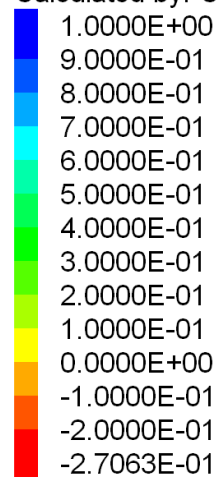


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Rock mass degradation behind a slope

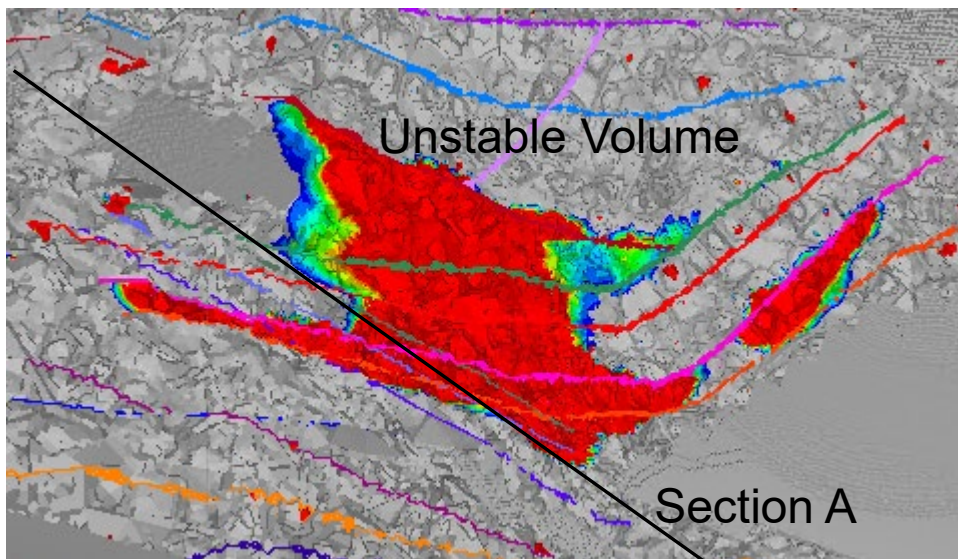
Zone Property emer_weak_sloss

Calculated by: Constant



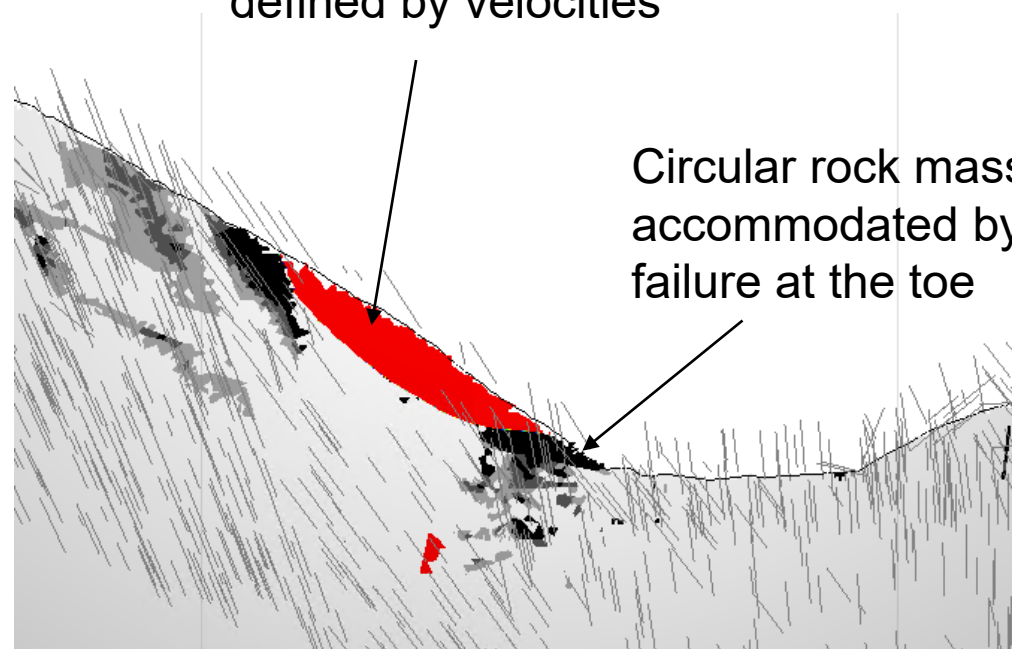
Rock mass degradation behind a slope

Potential failure of the slope
(volume in red)



Unstable Volume
defined by velocities

Circular rock mass failure
accommodated by UJ
failure at the toe



Section A

Some other important components of *IMASS*

- Density adjustment based on zone-based volumetric strain
- Weakness planes defined by Ubiquitous Joints
- Option between one or two residual envelopes
- Tracking stresses at failure
- *IMASS* uses the latest Itasca constitutive model framework and techniques for apex correction, automatic testing, and property update and management.

Installation and usage

- *IMASS* is built into *FLAC3D 7.0* and *3DEC 7.0*
- Invoking the model:

model config imass

zone cmodel assign imass

zone initialize density [_density]

zone property in_stren_gsi [_gsi]

zone property in_stren_ucsi [_ucs]

zone property in_stren_mi [_mi]

zone property in_mod_youngintact [_young]

zone property in_weak_multecrit [_mult]

These are the only properties required to have all default behaviors active

Table 2: IMASS material property names

	Flags/Inputs	Calculated	Emergent
Peak strength	flag_stren_mod(off)	calc_stren_global	
	in_stren_gsi	calc_stren_tension	
	in_stren_mi	calc_stren_ucsrn	
	in_stren_ucsi	calc_stren_mb calc_stren_s calc_stren_a	
Modulus	in_mod_youngintact	calc_mod_youngrm calc_mod_poisson	emer_mod_young
Strength weakening	flag_weak_barton(on)	calc_weak_ecrit	emer_weak_sloss
	flag_weak_tencut(on)	calc_weak_zsize	emer_weak_esplastic
	in_weak_multecrit		emer_weak_istenweak
	in_weak_chmbr(4.33)		emer_stren_mcfrc
	in_weak_char(1.0)		emer_stren_mccoh
	in_weak_phib(30)		emer_stren_mcten
	in_weak_sloss(1.0)		
	in_weak_tabletension		
Dilation/ bulking & modulus softening	flag_bulking_denadj		emer_bulking_mcdil
	flag_bulking_modsoft		emer_bulking_den
	in_bulking_den		emer_bulking_totvsi
	in_bulking_dil		
Stress at failure	in_bulking_targetvsi		
	in_bulking_maxtotvsi(2/3)		
Ubiquitous joint	flag_sfai_failtrack(off)		emer_sfai_pmax emer_sfai_plungepmax emer_sfai_trendpmax emer_sfai_pmid emer_sfai_plungepmid emer_sfai_trendpmid emer_sfai_pmin emer_sfai_plungepmin emer_sfai_trendpmin emer_uj_esplastic
	flag_uj_usejoints(on)		
	in_uj_jcoh		
	in_uj_jdil		
	in_uj_jfrc		
	in_uj_jten		
	in_uj_jdip		
	in_uj_jddirection(on)		
	in_uj_jnx		
	in_uj_jny		
	in_uj_jnz		

Research and improvements

- IMASS is a constitutive model based on empirical relationships, its formulation is ever-evolving with the state-of-the-art knowledge of strength and post-peak behavior of brittle rock masses. The current focus on refinement of the IMASS behavior include:
 - A more robust criteria for estimation of critical plastic shear strain (post-peak brittleness)
 - Characterization of the upper- and lower-bound for the equivalent roughness (R) (especially for porosities below 15%)
 - Refinement of the dilation model consistent with the upper- and lower-bound R and how it transitions between those values with increasing rock mass porosity

Final remarks

- *IMASS* has been developed to represent the rock-mass response to stress changes using strain-dependent properties that are adjusted to reflect the impacts of dilation and bulking as a rock mass undergoes plastic deformation.
- The two-mode softening in *IMASS* allows for mobilization of a high apparent friction angle at low confinement when the fragments are formed in the rock mass. This followed by reduction in friction angle as the rock mass bulks allow for a realistic simulation of the rock mass post-peak behavior.
- *IMASS* and its predecessor, *CaveHoek*, have been developed and refined over the past decade with mining applications being their core purpose. They have been used successfully by Itasca on numerous operations and projects.

Questions & Answers

Learn more about *IMASS* at www.itascainternational.com/software/imass

